

563163

**Sandia National Laboratories  
Compliance Monitoring  
Parameter Assessment  
For 2014**

WBS 4.3.1

December 23, 2014

Prepared for the United States Department of Energy  
Carlsbad Field Office




**Sandia National Laboratories**

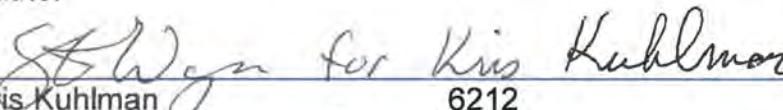
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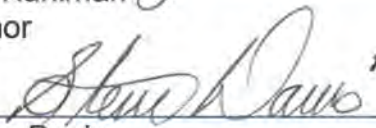
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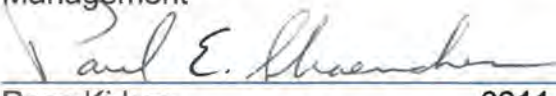
WBS 4.3.1  
Pkg. No. 510062

  
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## Executive Summary

This document reports the fifteenth annual derivation and assessment of the Waste Isolation Pilot Plant (WIPP) Compliance Monitoring Parameters (COMPs). The COMPs program is designed to meet certain requirements of the U.S. Environmental Protection Agency's (EPA) long-term disposal regulations (EPA 1993 and 1996). The concept of deriving and assessing COMPs is explained in Sandia National Laboratories (SNL) Activity/Project Specific Procedure, SP 9-8, titled: *Monitoring Parameter Assessment Per 40 CFR 194.42* (SNL 2011).

It is recognized that the data cut-off for information in this report is prior to the WIPP Operational accidents of February 2014, and as such, these events are not mentioned in any way in this document.

The WIPP has many monitoring programs, each designed to meet various regulatory and operational safety requirements. The comprehensive WIPP monitoring effort is not under the auspice of one program, but is comprised of many discrete elements, one of which was designed to fulfill the EPA's long-term disposal requirements found at 40 CFR Part 191 Subparts B and C, and the Certification Criteria at 40 CFR Part 194. Monitoring parameters that are related to the long-term performance of the repository were identified in a monitoring analysis.<sup>1</sup> Since these parameters fulfill a regulatory function, they were termed Compliance Monitoring Parameters so that they would not be confused with similar performance assessment (PA) input parameters.

The Department of Energy (DOE) uses PA to predict the radioactive waste containment performance of the WIPP. COMPs are used to indicate conditions that are not within the PA data ranges, conceptual model assumptions or expectations of the modelers and to alert the project of conditions not accounted for or anticipated. COMPs values and ranges were developed such that exceedance of an identified value indicates a condition that is potentially outside PA expectations. These values were appropriately termed "trigger values." Deriving COMPs trigger values (TVs) was the first step in assessing the monitoring data. TVs were first derived in 1999 and some have since been revised. The derivations and revisions are documented in the *Trigger Value Derivation Report* (Wagner and Kuhlman 2010).

In the initial Certification Ruling (EPA 1998a), EPA approved 10 COMPs, 2 relating to human activities, 5 relating to geotechnical performance, 2 relating to regional hydrogeology and 1 relating to the radioactive components of the waste. The requirements of 40 CFR § 194.4(b)(3) require the DOE to report any condition that would indicate the repository would not function as predicted or a condition that is substantially different from the information contained in the most recent compliance application. The DOE complies with these EPA requirements by conducting periodic assessments of COMPs that monitor the predicted performance of the repository and reporting any condition adverse to the containment performance. This compliance monitoring program is described in greater detail in DOE's *Compliance Monitoring Implementation Plan for 40 CFR §191.14(b), Assurance Requirements* (DOE 2014a)

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<sup>1</sup> Attachment MONPAR to Appendix MON in the CCA (DOE 1996) documents the analysis of monitoring parameters. The analysis was performed to fulfill 40 CFR § 194.42 requirements.

This 2014 COMPs assessment presents the results and recommendations based on the COMP monitoring data gathered during the annual reporting cycle. This assessment concludes that monitoring results are within expectations and no additional activities are warranted at this time.

# 1 Introduction

The WIPP is governed by the EPA's long-term radioactive waste disposal regulations at 40 CFR Part 191 Subparts B and C (EPA 1993) and the WIPP-specific certification criteria at 40 CFR Part 194 (EPA 1996). Monitoring WIPP performance is an "assurance requirement" of these regulations and is intended to provide additional confidence that the WIPP will protect the public and environment (see 40 CFR § 191.14). In the WIPP Compliance Certification Application (CCA; DOE 1996), the DOE made commitments to conduct a number of monitoring activities to comply with the criteria at 40 CFR § 194.42 and to ensure that deviations from the expected long-term performance of the repository are identified at the earliest possible time. These DOE commitments are represented by 10 COMPs, which are listed in Section 2.

The COMPs are an integral part of the overall WIPP monitoring strategy. The DOE's *Compliance Monitoring Implementation Plan for 40 CFR §191.14(b), Assurance Requirements* (MIP; DOE 2014a) describes the overall monitoring program and responsibilities for COMPs derivation and assessment. This report documents the results of the reporting year 2014 COMPs assessment (July 1<sup>st</sup> 2013 to June 30<sup>th</sup> 2014). This period matches the reporting period of the annual report that addresses 40 CFR § 194.4(b)(4) requirements (EPA 1996). This COMPs assessment follows the program developed under the original certification baseline using data and PA results from the current certified baseline, the 2009 Performance Assessment Baseline Calculation (PABC-2009). Although the CRA-2014 has been submitted to the EPA during this reporting period, the EPA has not completed their review and approval of the CRA-2014 PA. It is recognized that the data cut-off for information in this report is prior to the WIPP Operational accidents of February 2014, and as such, these events are not mentioned in any way in this document.

## 1.1 Monitoring and Evaluation Strategy

The Compliance Monitoring Program is an integrated effort between the Management and Operating Contractor (M&OC), the Scientific Advisor and the DOE Carlsbad Field Office (CBFO). The CBFO oversees and directs the monitoring program to ensure compliance with the EPA monitoring and reporting requirements. The Scientific Advisor (currently Sandia National Laboratories) is responsible for annually assessing COMPs and the development and maintenance of the trigger values (TVs). An observation beyond the acceptable range of TVs represents a condition that requires further actions, but does not necessarily indicate an out-of-compliance condition. This approach assures that conditions that are not consistent with expected repository performance are recognized as early as possible. These conditions may include data inconsistent with parameters, modeling assumptions or conceptual models implemented within PA, or conditions inconsistent with assumptions and arguments used in the screening of Features, Events and Processes (FEPs) for PA.

## 1.2 Reporting Cycle

The types of changes that must be reported to EPA are defined in 40 CFR §194.4. Under 40 CFR § 194.4, changes that differ from the activities or conditions outlined in the latest compliance application are defined as either significant or non-significant based on their potential impact on the compliance baseline and potential impact on containment performance. This part of the rule

also identified the timeframe to which the DOE is required to report significant and non-significant changes to the EPA. As such, the CCA states (Section 7.2.1) and subsequent recertification applications state that the results of the monitoring program will be submitted annually (DOE 1996, DOE 2004, DOE 2009, DOE 2014b). Additionally, the recertification requirements at 40 CFR §194.15(a)(2) also require inclusion of all additional monitoring data, analysis and results in the DOE's documentation of continued compliance as submitted in periodic Compliance Recertification Applications (CRAs). Monitoring data, the associated parameter values and monitoring information must be reported even if the assessment concludes there is no impact on the repository. The annual monitoring data will be compiled and provided to the DOE to fulfill DOE's monitoring reporting requirements to the EPA. The Scientific Advisor's role in the annual reporting task is to use the monitoring data to derive the COMPs (as necessary), compare the results to repository performance expectations in PA (annually), and to use the new and updated information to make any recommendations for modification to the Compliance Baseline, if merited.

## 2 Assessment of COMPs

The compliance monitoring program tracks the following 10 COMPs:

1. Probability of Encountering a Castile Brine Reservoir
2. Drilling Rate
3. Subsidence
4. Creep Closure
5. Extent of Deformation
6. Initiation of Brittle Deformation
7. Displacement of Deformation Features
8. Changes in Culebra Groundwater Flow
9. Change in Culebra Groundwater Composition
10. Waste Activity

A periodic review of these COMPs is necessary to meet the intent of 40 CFR §191.14 assurance requirements, which states:

“(b) Disposal systems shall be monitored after disposal to detect substantial and detrimental deviations from expected performance. This monitoring shall be done with techniques that do not jeopardize the isolation of the wastes and shall be conducted until there are no significant concerns to be addressed by further monitoring.”

This section summarizes the results of the 2014 assessment. In the following sections, each COMP is evaluated and compared to the applicable TV. This assessment is performed under Specific Procedure SP 9-8 (SNL 2011). A table for each of the 10 COMPs is used to summarize the evaluation and shows the COMP derivation, related PA parameters and FEPs, the current value for the COMPs as applicable and the TV.

### 2.1 Human Activities COMPs

The CCA identifies 10 COMPs that the DOE is required to monitor and assess during the WIPP operational period. Two of these parameters monitor “Human Activities” in the WIPP vicinity which include:

- Probability of Encountering a Castile Brine Reservoir
- Drilling Rate

### 2.1.1 Probability of Encountering a Castile Brine Reservoir

Table 2.1 summarizes data and TV information related to the COMP Probability of Encountering a Castile Brine Reservoir, as well as its implementation in PA. Monitoring activities for Castile brine encounters have identified no brine encounters during this reporting period. The total number of encounters identified since the CCA is 7. These encounters are detailed in Table 2.2. Data used for the CCA were compiled from drilling record searches for the region surrounding the WIPP up to 1995. The results of this initial search recorded 27 drilling encounters with pressurized brine (water) in the Castile Formation. Of these encounters, 25 were hydrocarbon wells scattered over a wide area in the vicinity of the WIPP site; 2 wells, ERDA 6 and WIPP 12, were drilled in support of the WIPP site characterization effort (see DOE 2014c, for a complete listing of brine encounters). The Delaware Basin Drilling Surveillance Program reviews the well files of all new wells drilled in the New Mexico portion of the Delaware Basin each year looking for encounters with pressurized Castile brine. Since the CCA, data have been compiled through August 2014. During this reporting period, no pressurized Castile brine encounters have been reported in the official drilling records for wells drilled in the New Mexico portion of the Delaware Basin (DOE 2014c).

Of the 7 Castile brine encounters recorded since the 1996 CCA, 6 were identified when WIPP Site personnel performing field work talked to area drillers. These encounters were inconsequential to the drilling process. The other encounter was reported by an operator in an annual survey of area drillers. All the new encounters are located in areas where Castile brine is expected to be encountered during the drilling process. Table 2.2 shows all known Castile brine encounters in the vicinity of the WIPP Site since the CCA.

The impacts of brine encounters are modeled in the PA. The CCA used a 0.08 probability of encountering a Castile brine reservoir. In the Performance Assessment Verification Test (PAVT), the EPA mandated a probability range of 0.01 to 0.60 (uniform distribution). The new range did not significantly influence the predicted performance of the repository. This range has been used in all PAs since the original WIPP certification with the exception of the CRA-2014. The CRA-2014 reanalyzed drilling data around the WIPP and proposed a different brine encounter probability. As stated previously, the EPA has not approved the PA used in this recertification application and therefore the PAVT brine encounter is still the baseline value. The mean of the baseline parameter is approximately 0.30. This value is significantly more than the 0.08 originally used in the CCA which was based on a geostatistical analysis of actual brine encounters. Results of more than 10 years of monitoring drilling encounters have shown that it is unlikely that further monitoring will show a probability near 0.30. The EPA also determined in their first certification sensitivity analysis that this parameter (PBRINE) does not have a significant impact on PA results (EPA 1998b).



**Table 2.1 Probability of Encountering a Brine Reservoir - 2014:**

<b>Trigger Value Derivation</b>				
<b>COMP Title:</b>	Probability of Encountering a Castile Brine Reservoir			
<b>COMP Units:</b>	Unitless			
<b>Related Monitoring Data</b>				
Monitoring Program	Monitoring Parameter ID	Characteristics (e.g., number, observation)	Compliance Baseline Value	
DBMP <sup>(1)</sup>	NA	Driller's survey – Field observations	0.01 to 0.60 (uniform distribution)	
<b>COMP Assessment Process</b>				
Analysis of encounters of pressurized brine recorded and reported by industry in the 9-township area centered on WIPP.				
<b>Year 2013 COMP Assessment Value - Reporting Period September 2013 to August 2014</b>				
No new data reported in State record during the reporting period; no new report from Field Observations. 34 Total Brine Encounters out of 678 boreholes drilled within the monitored area 27 CCA total occurrences before 1996 0 State Record occurrences since 1996 7 Site Personnel/ Drillers Survey occurrences since 1996				
<b>Related Performance and Compliance Elements</b>				
Element Title	Parameter Type & ID or Model Description	Derivation Procedure	Compliance Baseline	Impact of Change
Probability of Encountering Brine	Parameter PRBRINE	CCA MASS Attachment 18-6 geostatistical study based on area occurrences.  EPA Technical Support Document justified the upper value in their range by rounding up the upper value interpreted from the Time Domain Electromagnetic survey, which suggested a 10 to 55% areal extent.	0.08  0.01 to 0.60	Not a sensitive parameter.
<b>Monitoring Data Trigger Values</b>				
Monitoring Parameter ID	Trigger Value	Basis		
Probability of Encountering a Castile Brine Reservoir	None	After the DOE proposed the brine reservoir probability as potentially significant in the CCA Appendix MONPAR, the EPA conducted analyses that indicate a lack of significant effects on performance from changes in this parameter. For this reason and since the parameter is evaluated for significant changes at least once annually, no TV is needed.		

(1) Delaware Basin Monitoring Program

**Table 2.2. Well Locations Encountering Brine since the CCA<sup>2</sup>.**

<b>Number</b>	<b>Location</b>	<b>Well Name and Location</b>	<b>Spud Date</b>	<b>Well Information</b>
1	T21S-R31E-Sec 35	Lost Tank "35" - State #4	09/11/2000	Oil Well: Estimated several hundred barrels per hour. Continued drilling.
2	T21S-R31E-Sec 35	Lost Tank "35" - State #16	02/06/2002	Oil Well: At 2,705 ft, encountered 1,000 barrels per hour. Shut-in to get room in reserve pit with pressure of 180 psi. and water flow of 450 barrels per hour. Two days later, no water flow/full returns.
3	T22S-R31E-Sec 2	Graham "AKB" State #8	04/12/2002	Oil Well: Estimated 105 barrels per hour. Continued drilling.
4	T23S-R30E-Sec 1	James Ranch Unit #63	12/23/1999	Oil Well: Sulfur water encountered at 2,900 ft. 35 ppm H <sub>2</sub> S was reported but quickly dissipated to 3 ppm in a matter of minutes. Continued drilling.
5	T23S-R30E-Sec 1	Hudson "1" - Federal #7	01/06/2001	Oil Well: Estimated initial flow at 400 to 500 barrels per hour with a total volume of 600 to 800 barrels. Continued drilling.
6	T22S-R30E-Sec 13	Apache "13" - Federal #3	11/26/2003	Oil Well: Encountered strong water flow with blowing gas at 2,850-3,315 ft. 362 ppm H <sub>2</sub> S was reported. Continued drilling.
7	T21S-R31E-Sec 34	Jaque "AQJ" - State #7	03/04/2005	Oil Well: Encountered 104 barrels per hour at 2,900 ft. No impact on drilling process.

<sup>2</sup> From DOE 2014a, Table 7

### 2.1.2 Drilling Rate

Table 2.3 summarizes data and TV information related to the COMP Drilling Rate parameter and its implementation in PA. The drilling rate COMP tracks deep drilling (> 2,150 ft in depth) activities relating to resource exploration and extraction. Boreholes relating to resources include potash and sulfur core-holes, hydrocarbon exploration wells, saltwater disposal wells and water wells drilled in the Delaware Basin. The first drilling rate, reported in the CCA, was determined using an equation provided in 40 CFR Part 194. The drilling rate formula is as follows:

$$D_r = (D_{100} \times 1,000 \text{ yrs}) \div A_{DB} \quad (1)$$

where

$D_r$  = Drilling Rate (boreholes per km<sup>2</sup> per 10,000 yrs)

$D_{100}$  = Deep boreholes greater than 2,150 ft depth drilled over the last 100 yrs

$A_{DB}$  = Area of the Delaware Basin (23,102 km<sup>2</sup>)

The rate reported in the CCA using this equation was 46.8 boreholes per square kilometer over 10,000 years. Including the time period after the CCA (June 1996 to August 2014) increases the rate to 77.6 boreholes per square kilometer per 10,000 years (DOE 2014c).

As shown in Table 2.4, the drilling rate has risen from 46.8 holes per square kilometer to 77.6 holes per square kilometer since 1996. As a result of continuing analysis and monitoring, the TV for this COMP was removed (Wagner and Kuhlman 2010). No additional actions are recommended at this time.

**Table 2.3 Drilling Rate - 2014:**

<b>COMP Title:</b>		Drilling Rate		
<b>COMP Units:</b>		Deep boreholes (i.e., > 2,150 ft deep)/square kilometer/10,000 years		
<b>Related Monitoring Data</b>				
<b>Monitoring Program</b>	<b>Monitoring Parameter ID</b>	<b>Characteristics (e.g., number, observation)</b>		
DBMP	Deep hydrocarbon boreholes drilled	Integer per year		
<b>COMP Assessment Process</b>				
(Total number of deep boreholes drilled/number of years of observations (100)) x (10,000/23,102) [i.e., over 10,000 years divided by the area of the Delaware Basin in square kilometers]				
<b>Year 2013 COMP Assessment Value - Reporting Period September 1, 2013 to August 31, 2014</b>				
(17,939 boreholes on record for the Delaware Basin) Drilling Rate = 77.6 boreholes per square kilometer per 10,000 yrs.				
<b>Related Performance and Compliance Elements</b>				
<b>Element Title</b>	<b>Parameter Type &amp; ID or Model Description</b>	<b>Derivation Procedure</b>	<b>Compliance Baseline</b>	<b>Impact of Change</b>
Drilling rate	Parameter LAMBDAD	COMP/10,000 years	5.98 E-03 per square kilometer per year (CRA-2009 PABC value)	Cuttings/cavings releases increase proportionally with the drilling rate. Doubling CRA drilling rate does not exceed compliance limit.
<b>Monitoring Data Trigger Values</b>				
<b>Monitoring Parameter ID</b>	<b>Trigger Value</b>	<b>Basis</b>		
Deep boreholes	None	Revision 2 of the TV Derivation Report (Wagner and Kuhlman 2010).		

**Table 2.4. Drilling Rates for Each Year since the CCA.**

<i>Year</i>	<b>Number of Boreholes Deeper than 2,150 ft</b>	<b>Drilling Rate (boreholes per square kilometer per 10,000 years)</b>
1996 (CCA Value)	10,804	46.8
1997	11,444	49.5
1998	11,616	50.3
1999	11,684	50.6
2000	11,828	51.2
2001	12,056	52.2
2002 <sup>3</sup>	12,219	52.9
2002 (revised)	12,139	52.5
2003	12,316	53.3
2004	12,531	54.2
2005	12,819	55.5
2006	13,171	57.0
2007	13,520	58.5
2008	13,824	59.8
2009	14,173	61.3
2010	14,403	62.3
2011	14,816	64.1
2012	15,558	67.3
2013	16,633	72.0
2014	17,937	77.6

## 2.2 Geotechnical COMPs

The CCA lists ten monitoring parameters that the DOE is required to monitor and assess during the WIPP operational period. Five of these parameters are considered “geotechnical” in nature and include:

- Creep Closure
- Extent of Deformation
- Initiation of Brittle Deformation
- Displacement of Deformation Features
- Subsidence

Data needed to derive and evaluate the geotechnical COMPs are available from the most recent annual Geotechnical Analysis Report (GAR; DOE 2014d) and the annual Subsidence Monument Leveling Survey (DOE 2013). Three of the geotechnical parameters lend themselves to quantification: creep closure, displacement of deformation features, and subsidence. In contrast, the extent of deformation and initiation of brittle deformation are qualitative or observational

<sup>3</sup> In Revision 3 of Delaware Basin Monitoring Annual Report (dated 2002), the drilling rate for 2002 was shown as 52.9, with 12,219 deep boreholes. It was later noted that 80 shallow wells in Texas were listed as being deep. Correcting the classification of the 80 boreholes resulted in a reduction of the drilling rate from 53.9 to 52.5 (DOE 2013a).

parameters.

The WIPP GARs have been available since 1983 and are currently prepared by the M&OC on an annual basis. The purpose of the GAR is to present and interpret geotechnical data from the underground excavations. These data are obtained as part of a regular monitoring program and are used to characterize current conditions, to compare actual performance to the design assumptions, and to evaluate and forecast the performance of the underground excavations during operations. Additionally, the GAR fulfills various regulatory requirements and through the monitoring program, provides early detection of conditions that could affect operational safety, data to evaluate disposal room closure, and guidance for design changes. Data are presented for specific areas of the facilities including: (1) Shafts and Keys, (2) Shaft Stations, (3) Northern Experimental Area, (4) Access Drifts, and (5) Waste Disposal Areas. Data are acquired using a variety of instruments including convergence points and meters, multipoint borehole extensometers, rock bolt load cells, pressure cells, strain gauges, piezometers and joint meters. All of the geotechnical COMPs involve analyses of deformations/displacements, so the most pertinent data derived from the GAR are convergence and extensometer data. The most recent GAR (DOE 2014d) summarizes data collected from July 2012 through June 2013.

Subsidence monitoring survey reports are also prepared by the M&OC on an annual basis and present the results of leveling surveys performed in 2013 for nine vertical control loops comprising approximately 15 linear miles traversed over the ground surface of the WIPP site. Elevations are determined for 48 current monuments and 14 National Geodetic Survey vertical control points using digital leveling techniques to achieve Second-Order Class II loop closures or better. The data are used to estimate total subsidence and subsidence rates in fulfillment of regulatory requirements. The most recent survey (DOE 2013) summarizes data collected between September and October of 2013.

Comparisons between available geotechnical COMP related data and the TVs allow evaluation of the most recent geotechnical observations for the COMPs program. The cited reports and programs provide a good evaluation of all observations where deviations from historical normal occurrences are recorded. This process, as engaged for COMPs assessments, not only focuses attention on monitored parameters, it allows for reassessment of the proposed TVs. Notable deviations are addressed in the GAR and other references, and are reexamined here in the context of COMPs and TVs.

Geotechnical COMPs can be derived from or related to the repository's operational safety monitoring program, which has been implemented to ensure worker and mine safety. By nature, changes in geotechnical conditions evolve slowly; however, they are monitored continuously and reported annually. Since pertinent data from the underground reflect slowly evolving conditions, relationships that correlate to geotechnical COMPs also evolve slowly. Therefore, geotechnical conditions warranting action for operational safety will become evident before such conditions would impact long-term waste isolation. Monitoring underground response allows continuing assessment of conceptual geotechnical models supporting certification. In effect, these annual comparisons of actual geotechnical response with expected response serve to validate or improve models.

### **2.2.1 Creep Closure**

Table 2.5 summarizes data and TV information related to the COMP parameter Creep Closure,

and its implementation in PA. The GAR compiles all geotechnical operational safety data gathered from the underground. The most readily quantifiable geomechanical response in the WIPP underground is creep closure. The GAR routinely measures and reports creep deformation, either from rib-to-rib, roof-to-floor, or extensometer borehole measurements. With the exception of newly mined openings, rates of closure are relatively constant within each zone of interest and usually range from about 1-5 cm/yr. A closure rate in terms of cm/yr can be expressed as a global or nominal creep rate by dividing the displacement by the room dimension and converting time into seconds. Nominally these rates are of the order of  $1 \times 10^{-10}$  /s and are quite steady over significant periods. From experience, increases and decreases of rates such as these might vary by 20 percent without undue concern. Therefore, the “trigger value” for creep deformation was set as one order of magnitude increase in creep rate. Such a rate increase would alert the M&OC geotechnical staff to scrutinize the area exhibiting accelerating creep rates.

Extensive GAR data suggest that a possible TV could be derived from creep rate changes. The WIPP underground is very stable, relative to most operating production mines, and deformation is steady for long periods. However, under certain conditions creep rates accelerate, indicating a change in the deformational processes. The coalescence of microfractures into an arch-shaped fracture (or macrofracture) that extends into (or intersects) an overlying clay seam might create the onset of the roof beam de-coupling and increase the measured closure rate. Phenomena of fracture coalescence and damaged rock zone (DRZ) growth comprise important elements of PA assumption confirmation. Therefore, a measured creep rate change over a yearly period constitutes the COMP TV for creep closure. Rate changes are necessarily evaluated on a case-by-case basis since closure is related to many factors such as age of the opening, location in the room or drift, convergence history, recent excavations, and geometry of the excavations.

The creep deformation COMP is addressed by examining the deformations measured in specific regions of the underground including: (1) Shafts and Shaft Stations and (2) Access Drifts and Waste Disposal Areas. Figure 2.1 shows the current configuration of the WIPP underground with specific elements and regions annotated for reference. Information used for all geotechnical COMPs is derived from the GAR which has a reporting period ending June 30, 2012. For this reporting period, Panels 1 through 7 had been fully excavated. Figure 2.1 shows all areas mined as of June 30, 2012. At that time, CH waste was being emplaced in Panel 6, Room 5. In Panels 1 through 5, waste disposal operations had ceased and the entry drifts had been sealed to prevent access (please note that the reporting period for geotechnical information is through June 2013 such that the reported mining and emplacement activities depicted in Figure 2.1 from the GAR are not as current as the waste activity COMP information, which is through June 30, 2014).

**Table 2.5 Creep Closure - 2014:**

<b>COMP Title:</b>	Creep Closure			
<b>COMP Units:</b>	Closure Rate ( $s^{-1}$ )			
<b>Related Monitoring Data</b>				
<b>Monitoring Program</b>	<b>Monitoring Parameter ID</b>	<b>Characteristics (e.g., number, observation)</b>	<b>Compliance Baseline Value</b>	
Geotechnical	Closure	Instrumentation located throughout the underground.	Multi-mechanism deformation creep model developed by Munson and Dawson	
<b>COMP Assessment Process - Reporting Period July 2012 through June 2013</b>				
Evaluate GAR for centerline closure rates, compare to previous year's rate. Account for drift dimensions and convert to creep rate. If closure rate increases by greater than one order of magnitude, initiate technical review.				
<b>Related Performance and Compliance Elements</b>				
<b>Element Title</b>	<b>Parameter Type &amp; ID or Model Description</b>	<b>Derivation Procedure</b>	<b>Compliance Baseline</b>	<b>Impact of Change</b>
Repository Fluid Flow	Creep Closure	Porosity Surface, waste compaction, characteristics, waste properties, evolution of underground setting	SANTOS, porosity surface calculations	Provides validation of the creep closure model.
<b>Monitoring Data Trigger Values</b>				
<b>Monitoring Parameter ID</b>	<b>Trigger Value</b>	<b>Basis</b>		
Creep Closure	Greater than one order of magnitude increase in closure rate.	The closure rate increase signals potential de-coupling of rock.		



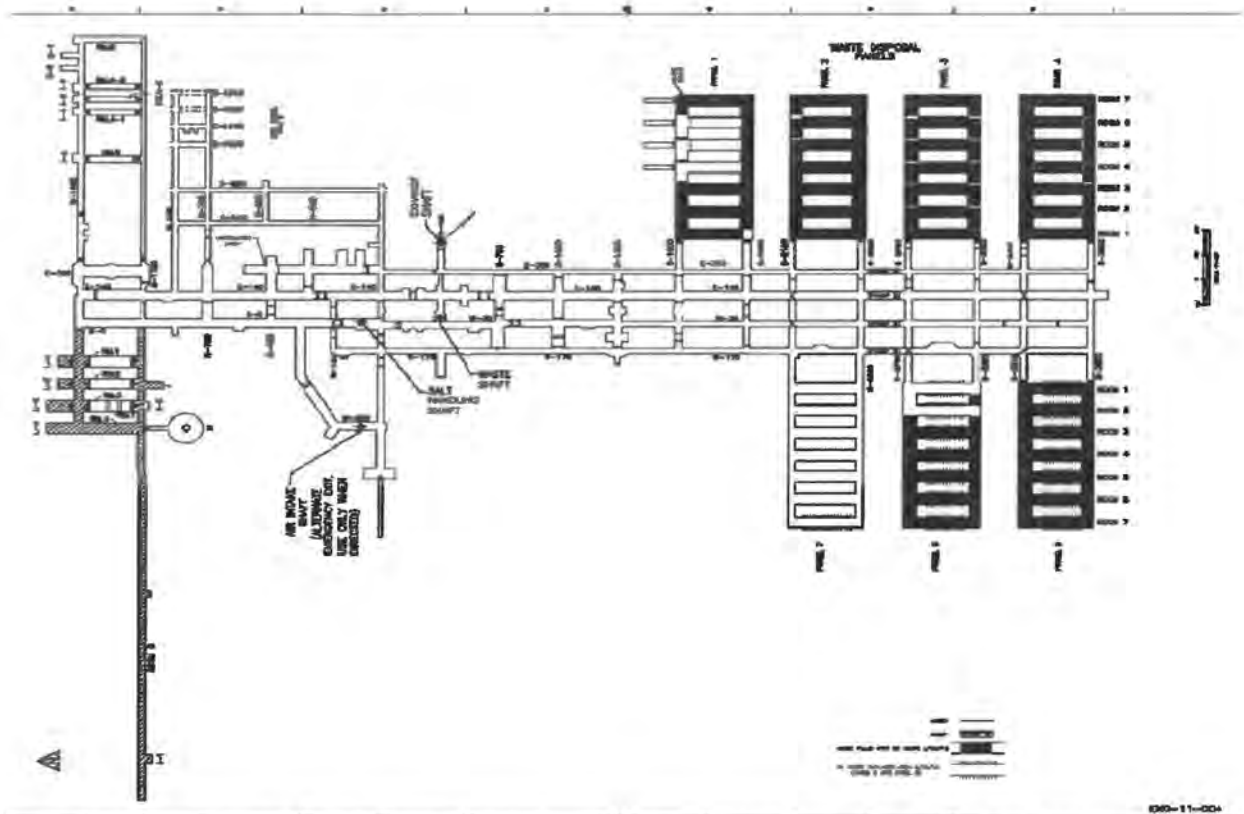


Figure 2.1. Configuration of the WIPP Underground for Geotechnical COMPs (after DOE 2014d; Reporting Period July 2012 through June 2013).

### *Shafts and Shaft Stations*

The WIPP underground is serviced by four vertical shafts including the following: (1) Salt Handling Shaft, (2) Waste Shaft, (3) Exhaust Shaft, and (4) Air Intake Shaft. At the repository level (approximately 2,150 ft below ground surface), enlarged rooms have been excavated around the Salt Handling and Waste Shafts to allow for movement of equipment, personnel, mined salt and waste into or out of the facility. The enlarged rooms are called shaft stations and assigned designations consistent with the shaft they service (e.g., Salt Handling Shaft Station).

Shafts. With the exception of the Salt Handling Shaft, the shafts are configured nearly identically. From the ground surface to the top of the Salado Formation, the shafts are lined with un-reinforced concrete. Reinforced concrete keys are cast at the Salado/Rustler interface with the shafts extending through the keys to the Salado. Below the keys, the shafts are essentially “open holes” through the Salado Formation and terminate either at the repository horizon or at sumps that extend approximately 40 m below the repository horizon. In the Salt Handling Shaft, a steel liner is grouted in place from the ground surface to the top of the Salado. Similar to the three other shafts, the Salt Handling Shaft is configured with a reinforced concrete key and is “open-hole” to its terminus. For safety purposes, the portions of the open shafts that extend through the Salado are typically supported using wire mesh anchored with rock bolts to contain rock fragments that may become detached from the shaft walls. Within the Salado Formation, the shaft diameters range from 3.65 m to 7.0 m.

Data available for assessing creep deformations in the salt surrounding the shafts are derived exclusively from routine inspections and extensometers extending radially from the shaft walls. These data are reported annually in the GAR. Piezometers and earth pressure cells were also

installed in the shafts during construction to monitor fluid pressure behind the shaft liners and key sections. The Salt Handling Shaft, Waste Shaft, and Air Intake Shaft are inspected weekly by underground operations personnel. Although the primary purpose of these inspections is to assess the conditions of the hoisting and mechanical equipment, observations are also made to determine the condition of the shaft walls, particularly with respect to water seepage, loose rock, and sloughing. In contrast to the other three shafts, the Exhaust Shaft is inspected quarterly using remote-controlled video equipment. These inspections have focused on salt build-up in the Exhaust Shaft and the impacts this build-up has on cabling and (currently not in use) air and water lines in the shaft. Based on these visual observations, all four shafts are in satisfactory condition and have required only routine ground-control activities during this reporting period.

Shortly after its construction, each shaft was instrumented with extensometers to measure the inward movement of the salt at three levels within the Salado Formation. In addition to COMPs assessment, measurements of shaft closure are used periodically as a calibration of numerical models and have been used in shaft seal system design. The approximate depths corresponding to the three instrumented levels are 330 m, 480 m and 630 m. Three extensometers are emplaced at each level to form an array. The extensometers comprising each array extend radially outward from the shaft walls and are equally spaced around the perimeter of the shaft wall. Over the years, most of these extensometers have malfunctioned. As a result, reliable data are not available at some locations. The DOE currently has no plans to replace failed instrumentation installed in any of the shafts because monitoring data acquired to date have shown no unusual shaft movements or displacements. It should be noted that no extensometer data was collected from the shafts during the reporting period because of a data logger failure. The type of extensometer used and its compatible data logger are no longer manufactured. DOE does not plan to replace the logger with an alternate because of compatibility and interface issues.

Shaft Station. Shaft station openings are typically rectangular in cross-section with heights ranging from approximately 4 to 6 m and widths ranging from 6 to 10 m. Over the life-time of the individual shaft stations, modifications have been made that have altered the dimensions of the openings. In the past, portions of the Salt Handling Shaft Station have been enlarged by removing the roof beam that extended up to anhydrite “b”. In the Waste Handling Shaft Station, the walls have been trimmed to enlarge the openings for operational purposes. No major modifications were performed at the shaft stations during this reporting period. Ground control, bolt replacement, bolt trimming and cable shoe anchor replacement were performed as routine maintenance.

The effects of creep on the shaft stations are assessed through visual observations and displacement measurements made using extensometers and convergence points. Because of the modifications made over the years, many of the original instrumentation has been removed or relocated. In addition, some instruments have malfunctioned or have been damaged and no longer provide reliable data. Displacement rates from existing and functional instrumentation listed in the GAR for the current reporting period (2012-2013) and the previous reporting period (2011-2012) are summarized in Table 2.6. Most of the measurements are for vertical closure. Based on shaft station convergence data, the current vertical displacement rates range from 0.10 to 1.90 in/yr (0.25 to 4.83 cm/yr). Dividing convergence rates by the average room dimension (approximately 6 meters) and expressing the results in units of 1/s yields vertical creep rates between approximately  $1.32 \times 10^{-11}/s$  to  $2.55 \times 10^{-10}/s$ . These rates are still low and represent typical creep rates for stable openings in salt. An examination of the percentage changes in displacement rates shown in Table 2.6 suggests the current shaft station displacement rates

(where available) are essentially identical to those measured during the previous reporting period. Based on the extensometer and convergence data, as well as the limited maintenance required in the shaft stations during the last year, creep deformations associated with the WIPP shaft stations are considered acceptable and meet the TV requiring creep deformation rates to change by less than one order of magnitude in a one-year period.

**Table 2.6. Summary of Closure Rates for WIPP Shafts and Shaft Stations.**

Location	Inst. Type <sup>(a)</sup>	Displacement Rate (in/yr)		Change In Rate (%)
		2011–2012	2012-2013	
Salt Handling Shaft	No extensometers remain functional			
Waste Handling Shaft	No extensometer data available for 2006-2012			
Exhaust Shaft	No extensometer data available for 2006-2012			
<b>Salt Handling Shaft Station</b>				
E0 Drift – S18 (A-E)	CP	2.50	1.70	-32
E0 Drift – S18 (B-D)	CP	2.60	1.90	-27
E0 Drift – S18 (F-H)	CP	1.50	1.10	-27
E0 Drift – S30 (A-C)	CP	2.30	1.70	-26
E0 Drift – S65 (A-C)	CP	1.50	1.20	-20
E0 Drift – S30	Ext	0.4	0.4	0
E0 Drift – S60	Ext	0.2	0.3	50
<b>Waste Shaft Station<sup>(b)</sup></b>				
S400 Drift – W30 (Vert. CL)	Ext	0.10	0.00	-100
S400 Drift – E32 (Vert CL)	Ext	0.30	0.30	0
S400 – E32 (Horizontal)	CP	1.20	1.40	17
S400 – E85 (Horizontal)	CP	1.20	1.40	17
<b>Air Intake Shaft Station<sup>(b)</sup></b>				
S65 Drift – W620 (Vert CL)	Ext	0.30	0.10	-67 <sup>(c)</sup>
N95 Drift – W620 (Vert CL)	Ext	0.20	0.30	-50 <sup>(c)</sup>

(a) Instrument Type: Ext = extensometer; CP = convergence point.

(b) CL = Centerline

(c) Anchor at maximum range

### Access Drifts and Waste Disposal Area

**Access Drifts.** The access drifts comprise the four major north-south drifts extending southward from near the Salt Handling Shaft to the entries into the waste disposal panels and several short cross-drifts intersecting these major drifts. The access drifts are typically rectangular in cross-section with heights ranging from 4.0 m to 6.4 m and widths ranging from 4.3 m to 9.2 m.

At the end of the current reporting period (July 2012 to June 2013), waste disposal was ongoing in Room 1 of Panel 6. Mining of Panel 7 was completed in January of 2013. Panels 3 and 4 were excavated at a slightly higher stratigraphic position (2.4 m) than either Panels 1 or 2. The roof of these panels coincides with Clay G. As such, Panels 1, 2, 7 and 8 will be at the original horizon and Panels 3, 4, 5 and 6 approximately 2.4 m higher in elevation (roof at Clay G). Trimming, scaling, floor-milling and rock-bolting operations were performed as necessary during the reporting period. To provide a more stable floor over the expected lifetime of Panel 7, a wide channel was excavated in the floor of the panel to remove stiff bedded polyhalite and a hard anhydride bed. These stiffer beds would resist the lateral stress caused by creep and bow

upward into the mined opening. The floor was backfilled with run-of-mine salt to the original floor height.

Assessment of creep deformations in the access drifts is made through the examination of extensometer and convergence point data reported annually in the GAR. Table 2.7 summarizes the vertical and horizontal displacement data reported in the most recent GAR (DOE 2014d). The table examines percentage changes between displacement rates measured during the current and previous annual reporting periods and breaks these percentage changes into ranges (e.g., <0% which includes negative values, 0 to 25%, 25 to 50%, etc.). The numbers shown in the tables represent the number of instrumented locations located on the drift vertically or horizontally that fall within the range of the indicated percentage change. In general, convergence rate accelerations continue to be minor in most locations. Other areas that have shown an increase in closure rates can be directly attributed to mining in Panel 7. The majority of the rate changes for the 2013 COMPs data were low or near zero. For this 2014 COMPs report, the majority of the data are still in the lower two ranges. As was done in the 2013 COMPs report, the convergence data and extensometer data were combined. The maximum displacement rates corresponding to these data for the current reporting period are given below:

Maximum Vertical Displacement Rates along Access Drifts:

15.75 cm/yr

Maximum Horizontal Displacement Rate along Access Drifts:

8.38cm/yr

Using a typical average drift dimension of 5 m and the maximum displacement rates shown above, the inferred maximum creep rate is approximately  $9.99 \times 10^{-10}$ /s. This rate is based on the maximum displacement which is not representative of the behavior of the system. This rate is nearly identical to last year's rate of  $1.16 \times 10^{-9}$ /s.

Creep deformations associated with the Access Drifts are acceptable and meet the TV requiring creep deformation rates to change by less than one order of magnitude in a one-year period. High displacement rates observed at a few locations have little effect on safety as geotechnical engineering provides continuous ground-control monitoring and remediation on an as-needed basis.

Waste Disposal Area: The Waste Disposal Area is located at the extreme southern end of the WIPP facility and is serviced by the access drifts described above. Eventually, the Waste Disposal Area will include eight disposal panels, each comprising seven rooms (the major north-south access drifts servicing the eight panels will also be used for waste disposal and will make up the ninth and tenth panels). Panel 1 was constructed in the late 1980s, Panel 2 constructed during the 1999-2000 time period, Panel 3 constructed during the 2002-2004 time period and the completion of Panel 4 during 2006. Mining for Panel 5 was completed in February of 2008 and Panel 6 was completed in April of 2010. Mining of Panel 7 began April 24, 2010 and was completed in January of 2013 (the end of the GAR reporting period). Figure 2.1 shows the state of waste emplacement and mining for the GAR reporting period.

The waste emplacement rooms are rectangular in cross-section with a height of 4 m and a width of 10 m. Entry drifts that provide access into the disposal rooms are also rectangular; the exhaust

entry to the panel has a height of 3.65 m and a width of 4.30 m while the air intake entry to the panel is 4.0 m by 6.0 m.

**Table 2.7. Summary of Changes in Vertical and Horizontal Displacement Rates of the WIPP Access Drifts and Waste Disposal Area Openings.**

Location	Number of Instrument Locations Where the Indicated Percentage Change has Occurred					
	Percentage Increase in Displacement Rate for Measurements Made During the 2011-2012 and 2012-2013 Reporting Periods					
	< 0%	0 – 25%	25 – 50%	50 – 75%	75 – 100%	100 – 200%
Access Drifts						
Vertical	125	130	18	4	2	1
Horizontal	33	103	6	0	1	0
Waste Disposal Area						
Panel 5						
Vertical	0	0	0	2	0	0
Panel 6						
Vertical	24	11	2	1	0	0
Horizontal	1	2	0	0	0	0
Panel 7						
Vertical	59	0	0	0	0	0

Assessment of creep deformation in the waste disposal area is made through the examination of extensometer and convergence point data reported annually in the GAR. Tables 2.6 and 2.7 (presented previously) summarize, respectively, the vertical and horizontal displacement data reported in the most recent GAR (DOE 2013d). Panels 1, through 5 were previously closed and are no longer accessible. Panel 5 was closed in July of 2011. No new convergence points or extensometers were installed during this reporting period. Twenty-three replacement convergence points were installed in the access drift to replace those disturbed during floor trimming. Table 2.7 examines percentage changes between displacement rates measured during the current and previous reporting periods. In addition, extensometer data are based only on displacements of the collar relative to the deepest anchor. Since most control points are vertical for the panels, only the vertical displacement rate is calculated. The maximum displacement rate corresponding to these data are given below.

Maximum Vertical Displacement Rates along Waste Disposal Area:

17.78 cm/yr

Using a nominal disposal-area-opening dimension of 8 m and the maximum displacement rates shown above, the inferred maximum creep rate is approximately  $7.05 \times 10^{-10}/s$ . This rate is less than last year's rate of  $8.26 \times 10^{-10}/s$  and is consistent with previous COMPs report rates. Maximum creep rates for the waste disposal areas are all associated with newer excavations. The rate used here was from Panel 6. It should be noted that there are no horizontal measurements for Panels 4 through 7. No additional actions are recommended at this time.

## 2.2.2 Extent of Deformation

Table 2.8 summarizes the data and TV information relating to the COMP parameter Extent of Deformation, as well as its implementation in PA. The extent of brittle deformation can have important implications to PA. As modeled in PA, the DRZ releases brine to the disposal room while properties of the DRZ control hydrologic communication between disposal panels. Therefore, extent of deformation is related to a conceptual model used in performance determinations. If characteristics could be tracked from inception, the spatial and temporal evolution of the DRZ would provide a validation benchmark for damage calculations.

Measurements in the GAR include borehole inspections, fracture mapping and borehole logging. These observations are linked closely to other monitoring requirements concerned with initiation of brittle deformation and displacement of deformation features. A more in-depth discussion of the condition of the mined areas is found in the “Ground Control Annual Plan for the Waste Isolation Pilot Plant, (DOE 2012a). This document discusses ground conditions and the operational monitoring program that is used to assess these conditions.

The Geotechnical Engineering Department at WIPP has developed a database that compiles back-fracturing data. The supporting data for the GAR (Volume 2, DOE 2014b) consists of plan and isometric plots of fractures. Fracture development is most continuous parallel to the rooms and near the upper corners. These fractures are designated “low angle fractures” relative to the horizontal axis. The original excavation horizon results in a 2.4 m-thick beam of halite between the roof and Clay Seam G. Low-angle fractures arch over rooms and asymptotically connect with Clay Seam G. Although the preponderance of monitoring information derives from the roof (back), buckling extends into the floor to the base of Marker Bed 139, which is located about 2 m below the disposal room floors. Fracture mapping thus far is consistent with expectations and tracks stress trajectories derived from computational work. At this time, a comprehensive model and supporting data for model parameters for damage evolution has not been developed for PA.

Excavation of Panel 3 raises the waste disposal panels by 2.4 m such that the roof of the disposal rooms will be coincident with Clay Seam G and the floor will be an additional 2.4 m above Marker Bed 139. This change will likely alter the typical fracture patterns observed to date and may cause subtle changes in how the DRZ develops. Effects of excavation to Clay Seam G have been evaluated by finite element analyses to assess possible impact to PA (Park and Holland 2003). Their modeling shows that the DRZ does not extend below MB139 at the new horizon, as it does at the original horizon. The rise in repository elevation otherwise causes no discernible change to the porosity surface used in PA. Data provided in the GAR suggest that brittle deformation extends at least 2.4 m (to Clay Seam G where present) and perhaps as much as 4.5 m (to Clay Seam H) above the roof of the WIPP openings. In addition, brittle deformation extends below the floor of the openings to at least the base of Marker Bed 139 (approximately 2 to 3 m).

Fracture maps provided in the 2014 GAR (DOE 2014d) were compared to maps in the previous year’s report. There are maps for only Panels 6 and 7 in the 2013 and 2014 GAR. The current GAR includes only maps for room 1 of Panel 6 and all of Panel 7. The fracture maps for Panel 6 are nearly identical to last year’s maps. The only reportable changes between the maps for Panel 7 occurred in room 2 (at S2360) where two existing spaced fractures near the rib connected over the timeframe between fracture mappings. A new 30 foot fracture is shown in Panel 7 South

2180 drift near W980 close to the rib. Since these features are close to the rib where stress is high, they are not unexpected.

**Table 2.8 Extent of Deformation - 2014:**

<b>COMP Title:</b>		Extent of Deformation		
<b>COMP Units:</b>		Areal extent (length, direction)		
<b>Related Monitoring Data</b>				
<b>Monitoring Program</b>	<b>Monitoring Parameter ID</b>	<b>Characteristics</b> (e.g., number, observation)	<b>Compliance Baseline Value</b>	
Geotechnical	Displacement	Meters	Not Established	
<b>COMP Assessment Process - Reporting Period July 2012 through June 2013</b>				
Extent of deformation is deduced from visual inspections and fracture mapping which are examined yearly for active cross sections. Anomalous growth is determined by yearly comparison.				
<b>Related Performance and Compliance Elements</b>				
<b>Element Title</b>	<b>Parameter Type &amp; ID or Model Description</b>	<b>Derivation Procedure</b>	<b>Compliance Baseline</b>	<b>Impact of Change</b>
DRZ Conceptual Model	Micro- and macro-fracturing in the Salado Formation	Constitutive model from laboratory and field databases.	Permeability of DRZ was originally assigned a constant value of $10^{-15} \text{ m}^2$ for the CCA; per EPA direction, a uniform distribution from $3.16 \times 10^{-13}$ to $3.98 \times 10^{-20} \text{ m}^2$ was used for all subsequent PAs	DRZ spatial and temporal properties have important PA implications for permeability to gas, brine, and two-phase flow.
<b>Monitoring Data Trigger Values</b>				
<b>Monitoring Parameter ID</b>	<b>Trigger Value</b>	<b>Basis</b>		
Fractures at depth	None	TV Derivation Report, Revision 2 (Wagner and Kuhlman 2010)		



### 2.2.3 Initiation of Brittle Deformation

Table 2.9 summarizes data and TV information relating to the COMP parameter Initiation of Brittle Deformation, as well as its implementation in PA. Initiation of brittle deformation around WIPP openings is not directly measured and is therefore a qualitative observational parameter. By nature, qualitative COMPs can be subjective and are not prone to the development of well-defined TVs. In addition, this COMP is not directly related to a PA parameter. Brittle deformation eventually leads to features that are measured as part of geotechnical monitoring requirements, such as the extent and displacement of deformation features. Initiation of brittle deformation is expected to begin immediately upon creation of an opening. The ongoing geotechnical program will help quantify damage evolution around WIPP openings. Initiation, growth and extent of the DRZ are important considerations for the operational period panel closures as well as compliance PA calculations. As stated previously, this COMP is qualitative and is not directly related to PA parameters.

**Table 2.9 Initiation of Brittle Deformation - 2014:**

<b>COMP Title:</b>		<b>Initiation of Brittle Deformation</b>		
<b>COMP Units:</b>		Qualitative		
<b>Related Monitoring Data</b>				
Monitoring Program	Monitoring Parameter ID	Characteristics (e.g., number, observation)	Compliance Baseline Value	
Geotechnical	Closure	Observational	Not Established	
<b>COMP Assessment Process - Reporting Period July 2011 through June 2012</b>				
Qualitative and pertinent to operational considerations. Captured qualitatively in association with other COMPs				
<b>Performance and Compliance Elements</b>				
Element Title	Parameter Type & ID or Model Description	Derivation Procedure	Compliance Baseline	Impact of Change
Not directly related to PA as currently measured	NA	NA	NA	NA
<b>Monitoring Data Trigger Values</b>				
Monitoring Parameter ID	Trigger Value	Basis		
Initiation of Brittle Deformation	None	Qualitative COMPs can be subjective and are not prone to the development of meaningful TVs.		

### 2.2.4 Displacement of Deformation Features

Table 2.10 summarizes data and TV information relating to the COMP parameter Displacement of Deformation Features, as well as its implementation in PA. The displacement of deformation

features primarily focuses on those features located in the immediate vicinity of the underground openings, e.g., mining-induced fractures and lithological units within several meters of the roof and floor. As discussed previously, fracture development is typically continuous sub-parallel to the surface of the openings and terminating near the corners. These fractures tend to propagate or migrate by arching over and under the openings and, thus are designated “low-angle fractures” relative to the horizontal axis. Typically, the fractures intersect or asymptotically approach lithologic units such as clay seams and anhydrite stringers. As a result, salt beams are formed. In the roof, the beams are de-coupled from the surrounding formation requiring use of ground support. In the floor, the beams sometimes buckle into the openings requiring floor milling and trimming. Lithologic units of primary interest are Clays G and H. These features are located approximately 2.4 m and 4.5 m respectively, above the roof of Panels 1, 2, 7 and 8. Marker Bed 139 (anhydrite) is located approximately 2 m below the floor of these panels. For Panels 3 through 6, the panels are mined up to Clay G. Clay H is therefore located 2.1 m above the roof of these panels and Marker Bed 139 is located approximately 4.4 m below the panel floors.

**Table 2.10 Displacement of Deformation Features - 2014:**

<b>COMP Title:</b>	Displacement of Deformation Features			
<b>COMP Units:</b>	Length			
<b>Related Monitoring Data</b>				
Monitoring Program	Monitoring Parameter ID	Characteristics (e.g., number, observation)	Compliance Baseline Value	
Geotechnical	Delta D/D <sub>0</sub>	Observational	Not established	
<b>COMP Assessment Process - Reporting Period July 2012 through June 2013</b>				
Observational – Lateral deformation across 148 boreholes in Panels 6 & 7.				
<b>Related Performance and Compliance Elements</b>				
Element Title	Parameter Type & ID or Model Description	Derivation Procedure	Compliance Baseline	Impact of Change
Not directly related to PA	N/A	N/A	N/A	N/A
<b>Monitoring Data Trigger Values</b>				
Monitoring Parameter ID	Trigger Value	Basis		
Borehole diameter closure	None	TV Derivation Report Revision 2 (Wagner and Kuhlman 2010)		

Monitoring of these deformation features is accomplished through visual inspection of observation boreholes (OBH) drilled from the openings through the feature of interest. In general, these boreholes are aligned vertically (normal to the roof and floor surfaces) because of the location and orientation of the fractures and lithological units of interest. All of the OBHs are 7.6 cm (3 in) in diameter, and many intersect more than one deformation feature. The ages of the OBHs vary from more than 20 years to recent.

The deformation features in OBHs are classified as: 1) offsets, 2) separations, 3) rough spots and 4) hang-ups. Of the four features, offsets are the principle metric for this COMP and are quantified by visually estimating the degree of borehole occlusion created by the offset. The

direction of offset along displacement features is defined as the movement of the stratum nearer the observer relative to the stratum farther from the observer. Typically, the nearer stratum moves toward the center of the excavation. Based on previous observations in the underground, the magnitude of offset is usually greater in boreholes located near the ribs as compared to boreholes located along the centerline of openings.

All of the observation holes associated with Panels 1 through 5 are no longer monitored. There are a total of 148 OBHs reported in the GAR. These OBHs are located in the panels, access drifts and the North End of the repository. Based on the current data available from the GAR, 35 OBHs were at least 50% occluded, 7 were 100% occluded. There were 35 OBHs that has no reported offset. Panel 6 has the greatest separations that are associated with Clay H and Anhydrite "a." Panel 6 has 43 holes that have fractures associated with anhydrite stringers in the lower portion (first 3 feet) of the roof beam where there were only 19 in the last year's report. In Panel 7, 40 of the 47 holes showed some offset where there were only 16 in the last COMPs report. In general, panels mined to Clay G show less offsets over time than the other panels as the number of fully occluded OBHs has decreased in the upper panels.

Displacement of deformation features has been useful for implementation of ground control alternatives (i.e., horizon change to Clay G). Displacement features complement observation of brittle deformation initiation and corroborate estimates of the extent of deformation.

### 2.2.5 Subsidence

Table 2.11 summarizes data and TV information relating to the COMP parameter Subsidence, as well as its implementation in PA. Subsidence is currently monitored via elevation determination of 48 existing monuments and 14 of the National Geodetic Survey's vertical control points. Approximately 15 miles of leveling was performed in 2012 for 9 control loops (see Figure 2-2). To address EPA monitoring requirements, the most recent survey results (DOE 2012a) are reviewed and compared to derived TVs. Because of the low extraction ratio and the relatively deep emplacement horizon (2,150 ft), subsidence over the WIPP is expected to be much lower and slower than over the local potash mines. Maximum observed subsidence over potash mines near the WIPP is 1.5 m, occurring over a time period of months to a few years after initial mining. In contrast, calculations show that the maximum subsidence predicted directly above the WIPP waste emplacement panels is 0.62 m assuming emplacement of CH-TRU waste and no backfill (Backfill Engineering Analysis Report [BEAR; WID 1994]). Further considerations, such as calculations of room closure, suggest that essentially all surface subsidence would occur during the first few centuries following construction of the WIPP, so the maximal vertical displacement rates would be approximately 0.002 m/yr (0.006 ft/yr). Obviously, these predicted rates could be higher or lower depending on mining activities as well as other factors such as time. Because the vertical elevation changes are very small, survey accuracy, expressed as the vertical closure of an individual loop times the square root of the loop length, is of primary importance. For the current subsidence surveys, a Second-Order Class II loop closure accuracy of  $8 \text{ mm} \times \sqrt{\text{km}}$  (or  $0.033 \text{ ft} \times \sqrt{\text{mile}}$ ) or better was achieved in all cases. This year's measured accuracy ranged from -0.007 to 0.005 ft/mile.

**Table 2.11 Subsidence - 2014:**

<b>COMP Title:</b>	Subsidence			
<b>COMP Units:</b>	Change in surface elevation in meters per year			
<b>Related Monitoring Data</b>				
<b>Monitoring Program</b>	<b>Monitoring Parameter ID</b>	<b>Characteristics (e.g., number, observation)</b>	<b>Compliance Baseline Value</b>	
Subsidence Monitoring Leveling Survey (SMP)	Elevation of 62 original monitoring monuments	Decimal (meters)	Not Established	
SMP	Change in elevation over year	Decimal (meters)	Not Established	
<b>COMP Assessment Process – 2014; Data acquired between August through October of 2013</b>				
Survey data from annual WIPP Subsidence Monument Leveling are evaluated. Elevations of 48 monitoring monuments in nine loops are compared to determine changes.				
<b>Related Performance and Compliance Elements</b>				
<b>Element Title</b>	<b>Parameter Type &amp; ID or Model Description</b>	<b>Derivation Procedure</b>	<b>Compliance Baseline</b>	<b>Impact of Change</b>
Subsidence	FEP [W-23]	Predictions are of low consequence to the calculated performance of the disposal system – based on WID (1994) analysis and EPA treatment of mining.	Maximum total subsidence of 0.62 m above the WIPP.	Predicted subsidence will not exceed existing surface relief of 3 m – i.e., it will not affect drainage. Predicted subsidence may cause an order of magnitude rise in Culebra hydraulic conductivity (CRA-2009 Appendix PA, Attachment SCR, Section SCR-6.3.1.4) – this is within the range of hydraulic conductivity modeled in PA. Predicted WIPP subsidence is below that predicted for the effects of potash mining (0.62 m vs.1.5 m; DOE 2009).
<b>Monitoring Data Trigger Values</b>				
<b>Monitoring Parameter ID</b>	<b>Trigger Value</b>	<b>Basis</b>		
Change in elevation per year	1.0 x 10 <sup>-2</sup> m (3.25 x 10 <sup>-3</sup> ft) per year subsidence	Based on the most conservative prediction by analyses referenced in the CCA.		

Several monuments have also been included in various annual surveys, but were not included in the current surveys because the monuments no longer exist or have been physically disturbed. Table 2.12 lists these monuments. Historically, the surveys were conducted by private companies under subcontract to DOE; however, since 1993, the WIPP M&OC has conducted the surveys using a set of standardized methods. Starting with the 2002 survey, the M&OC has been following WIPP procedure WP 09-ES4001 (WTS 2002).

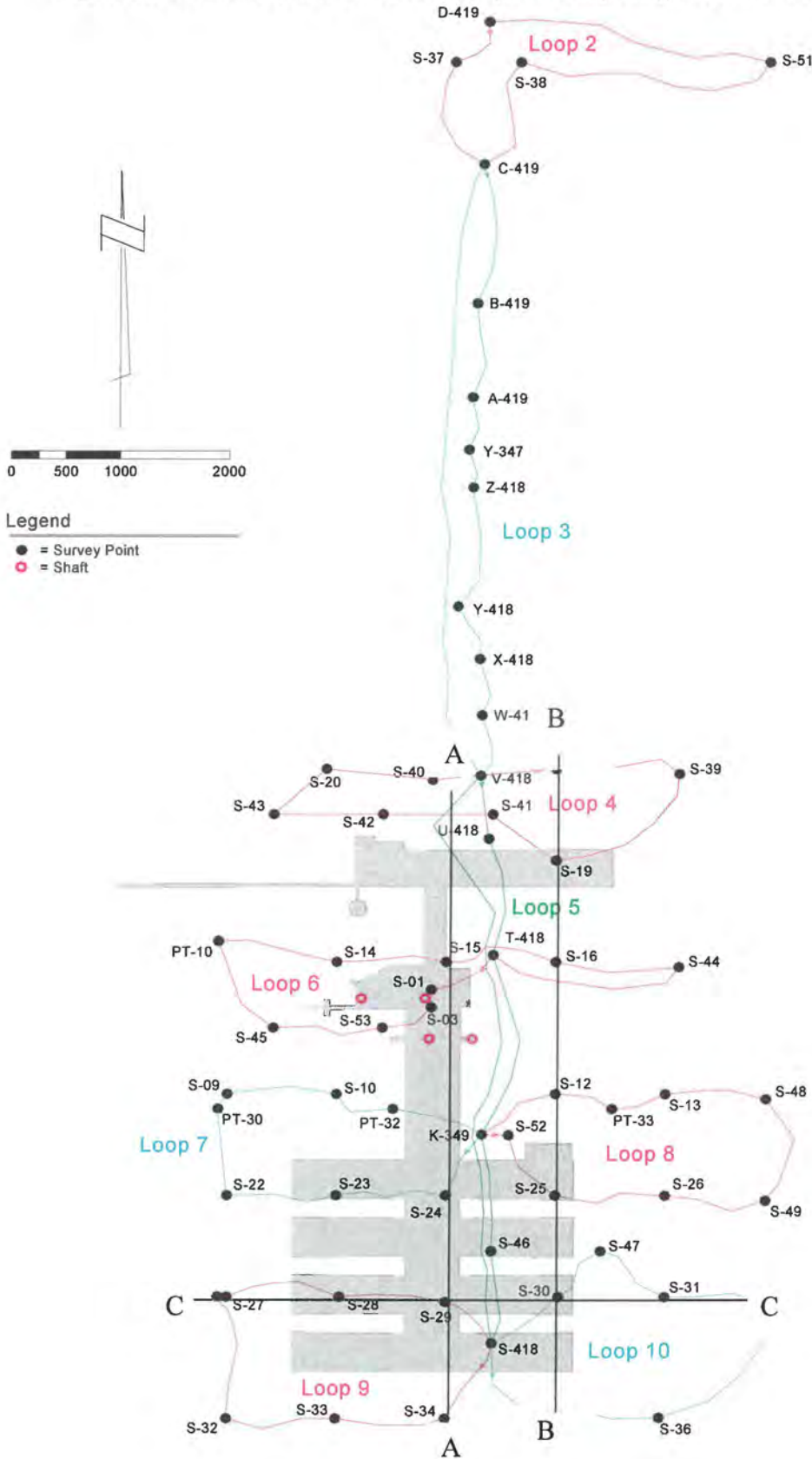
**Table 2.12 Monuments No Longer Surveyed**

Monument	Fate	Date Last Surveyed
S-02	No longer exists	1992
S-11	No longer exists	1992
S-17	No longer exists	2003
S-18	No longer exists	2003
S-54	No longer exists	1992
PT-30	Physically Disturbed	1997
PT-31	Physically Disturbed	2003

The current surveys comprise nine leveling loops containing as few as 5 to as many as 10 monuments/control points per loop as shown in Figure 2.2 (Surveys of Loop 1 benchmarks have been discontinued because only two benchmarks comprise this loop and these benchmarks are redundant to other survey loops). Elevations are referenced to Monument S-37 located approximately 7,700 ft north of the most northerly boundary of the WIPP underground excavation. This location is considered to be far enough from the WIPP facility to be unaffected by excavation-induced subsidence expected directly above and near the WIPP underground. The elevation of S-37 has been fixed at 3,423.874 ft for all of the subsidence leveling surveys conducted since 1993. Survey accuracy for all loops was within the allowable limits (DOE 2013). Adjusted elevations are determined for every monument/control point by proportioning the vertical closure error for each survey loop to the monuments/control points comprising the loop. The proportions are based on the number of instrument setups and distance between adjacent points within a survey loop.

The adjusted elevations for each monument/control point are plotted as functions of time to assess subsidence trends. Figures 2.3 through 2.7 provide, respectively, elevations for selected monuments including those located (1) directly above the first waste emplacement panel, (2) directly above the second waste emplacement panel, (3) directly above the north experimental area, (4) near the salt handling shaft, and (5) outside the repository footprint of the WIPP underground excavation. As expected, subsidence is occurring directly above the underground openings (Figures 2.3 through 2.6); however the magnitude of the subsidence above the repository is small ranging from about -0.003 ft to -0.427 ft.

Figure 2.2. Monuments and vertical control points comprising WIPP subsidence survey loops.



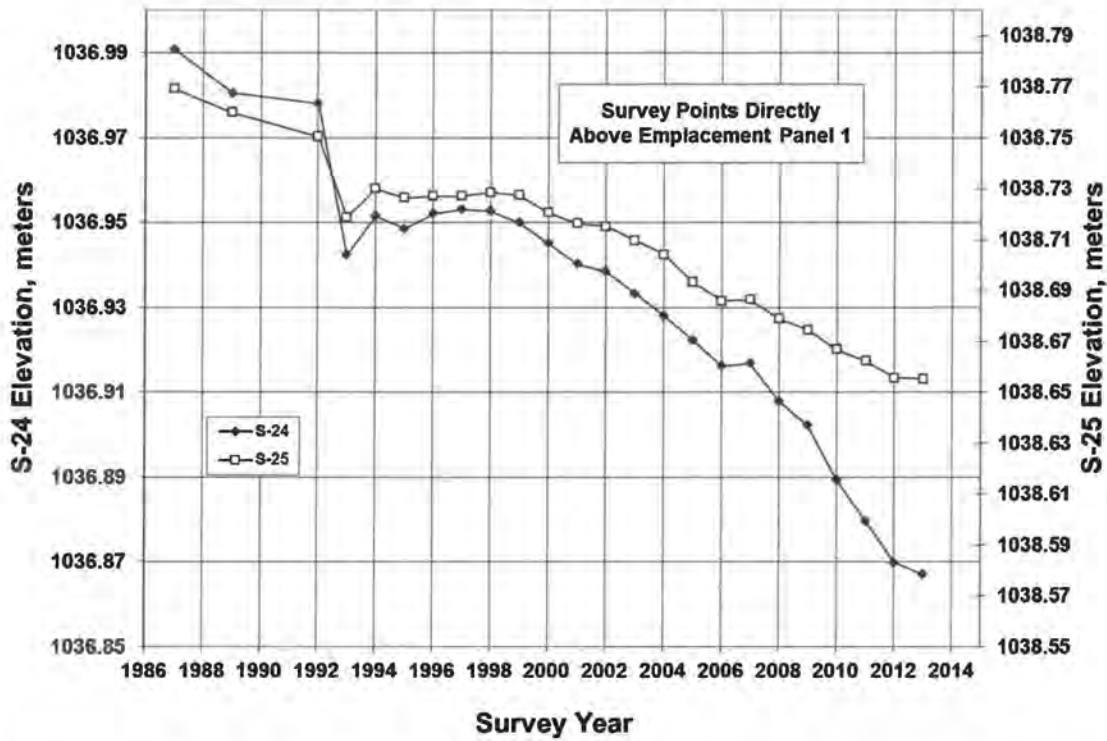


Figure 2.3. Elevations of WIPP monuments S-24 and S-25 located directly above emplacement Panel 1.

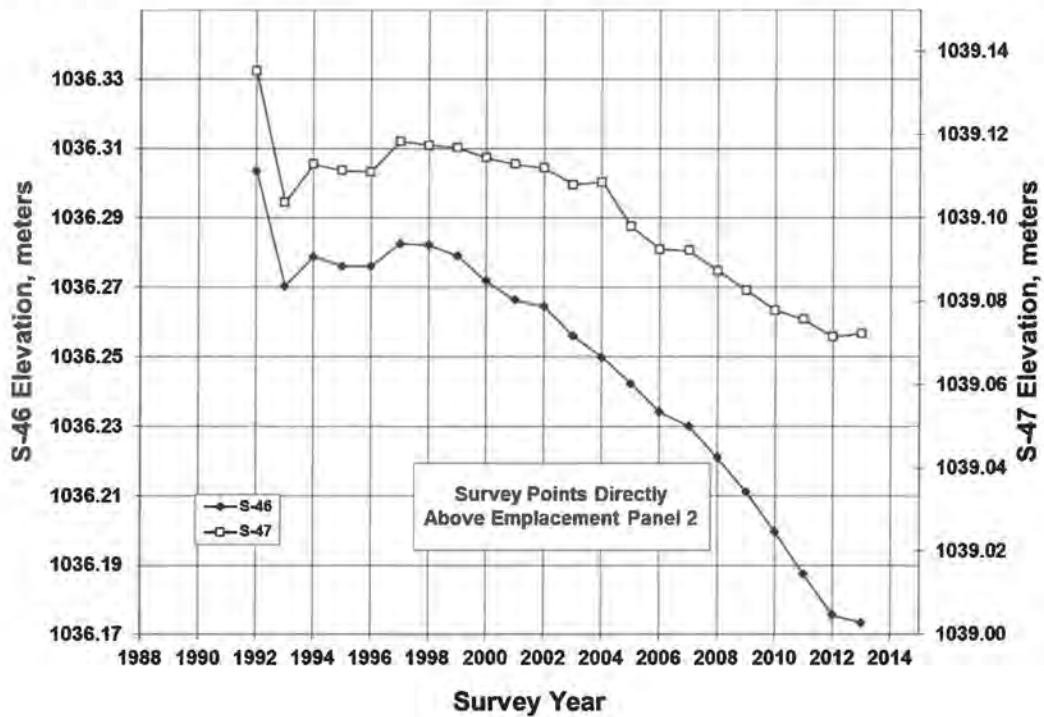


Figure 2.4. Elevations of WIPP monuments S-46 and S-47 located directly above emplacement Panel 2.

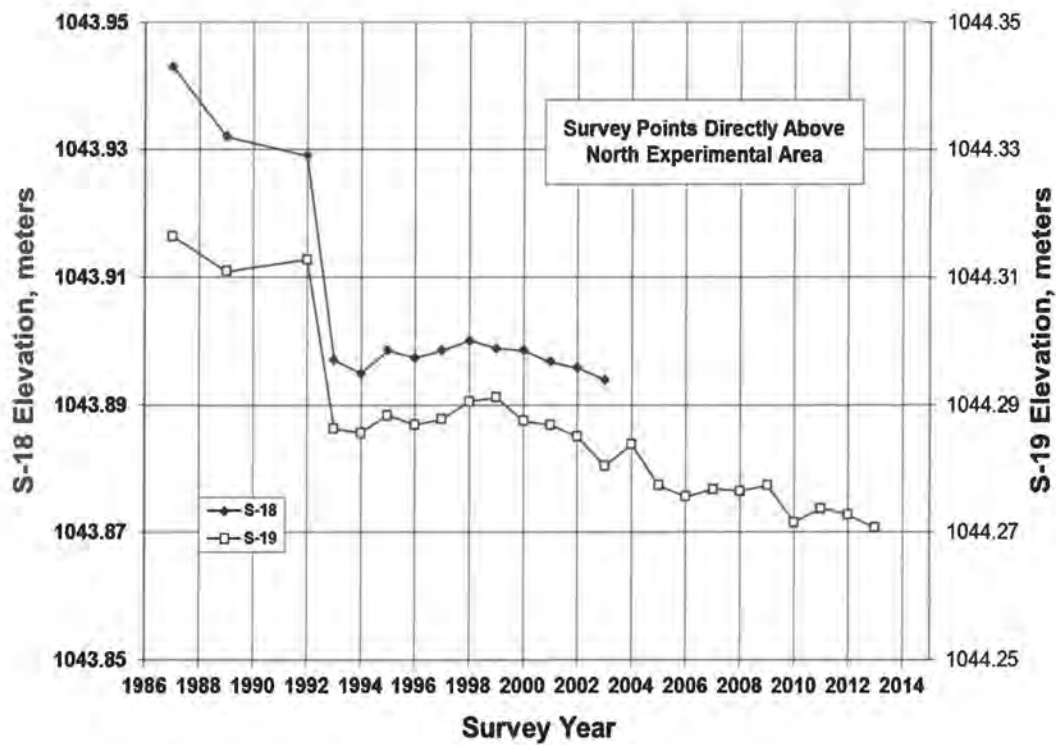


Figure 2.5. Elevations of WIPP monuments S-18 and S-19 located directly above the north experimental area.

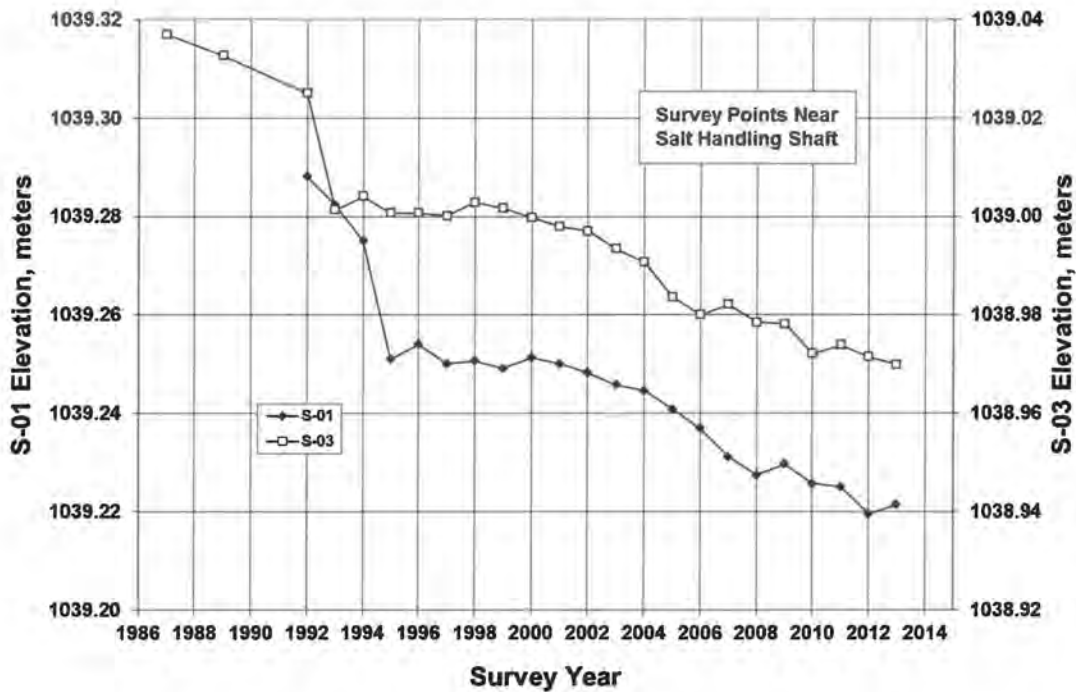


Figure 2.6. Elevations of WIPP monuments S-01 and S-03 located near the Salt Handling Shaft.



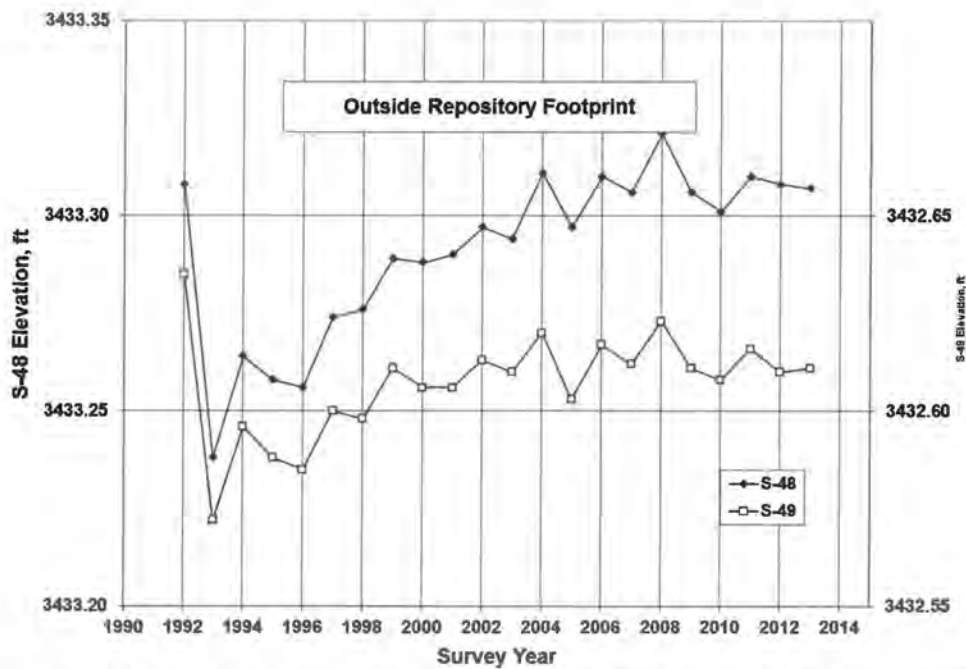


Figure 2.7. Elevations of WIPP monuments S-48 and S-49 located outside the repository footprint.

As time passes, subsidence is expected to be most pronounced directly above the WIPP underground excavations and will be minimal away from the repository footprint. Early results suggest this pattern is already occurring, as shown in Figures 2.8 through 2.10 for the following subsidence profiles (shown in plan view in Figure 2.2):

- Section A-A', North-South section extending through the WIPP site
- Section B-B', North-South section extending from the north experimental area through the south emplacement panels
- Section C-C', East-West section extending through Panel 1

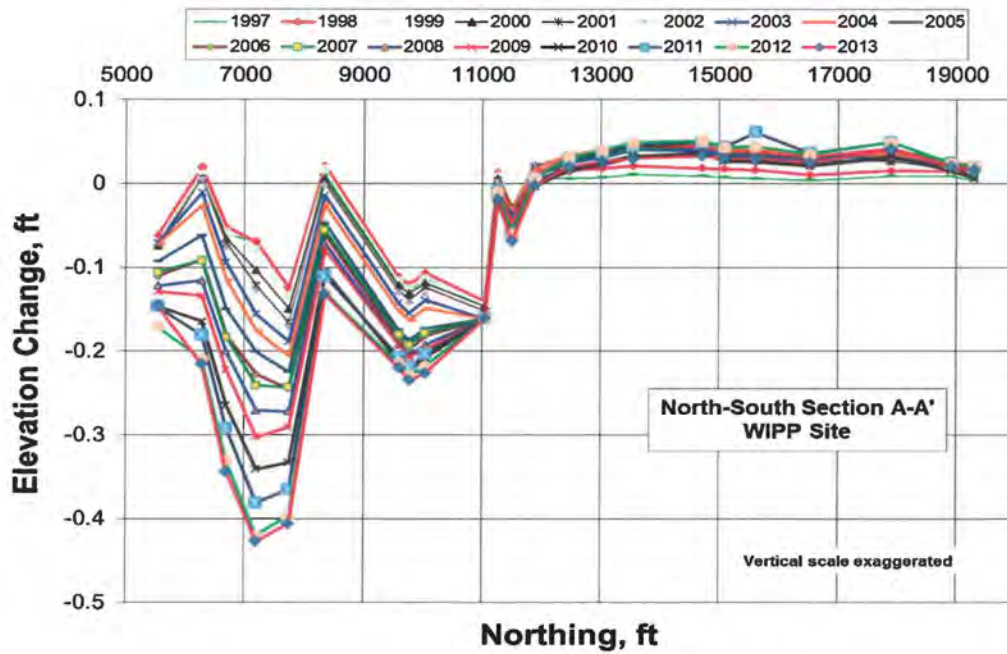


Figure 2.8. North-South subsidence profile A-A'.

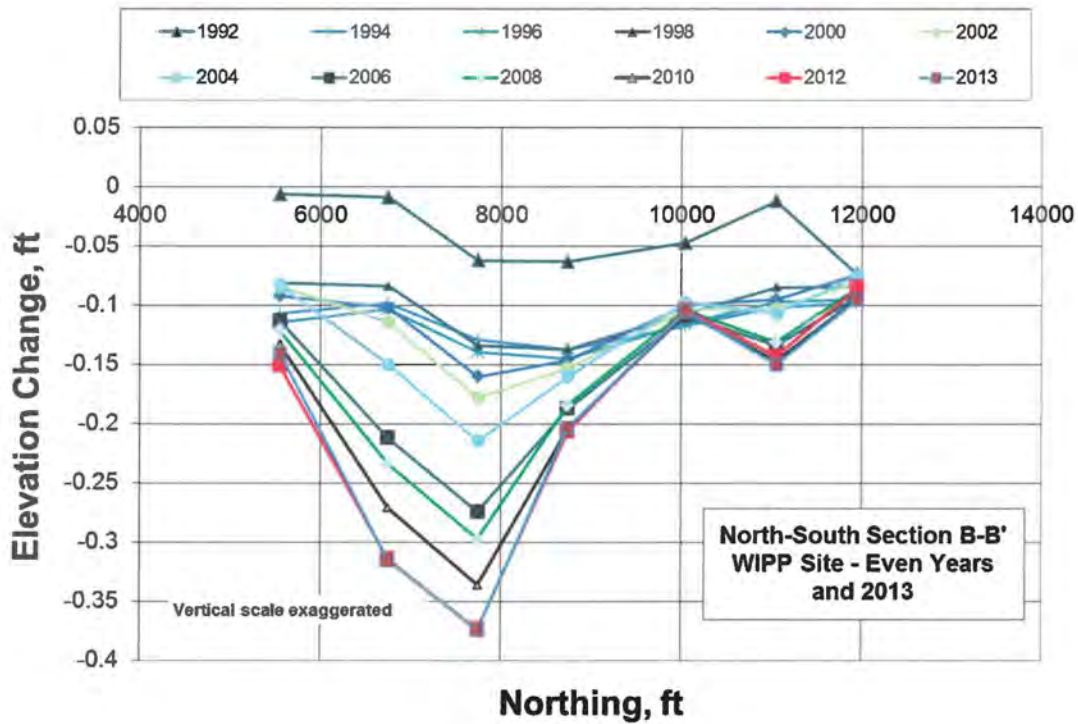


Figure 2.9. North-South subsidence profile B-B'.

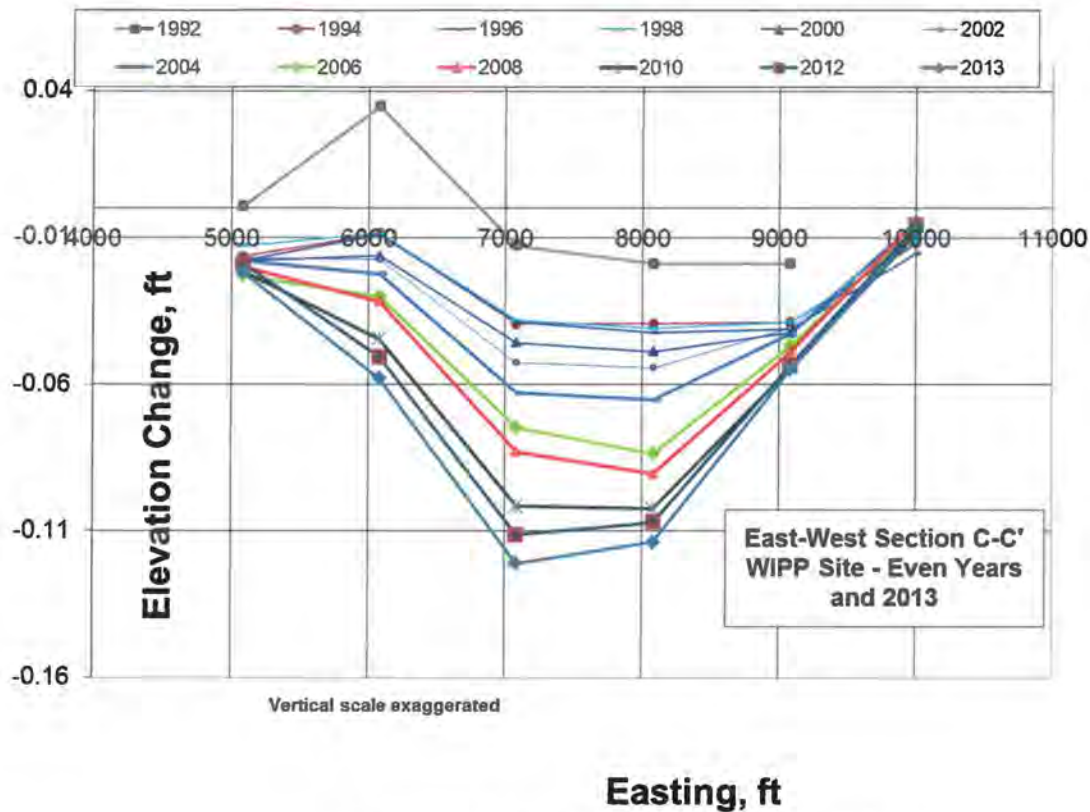


Figure 2.10. East-West subsidence profile C-C'.

The elevation changes of individual monuments shown in these figures are referenced to the elevations determined from the annual surveys that first incorporated the monument so, in some cases, direct temporal comparisons between pairs of monuments cannot be made. For example, only 29 monuments were included in the 1987 survey, while 50 monuments were included in the 1992 surveys and more than 60 for all surveys since 1996. Although direct comparisons cannot always be made, several observations for this reporting period are possible including:

1. The most significant total subsidence (greater than - 0.25 ft) occurs above the waste panels (Monuments S-01, S-23, S-24, S-25, S-28, S-29, S-30, S-46 and PT-32). This subsidence trend is centered over Panels 1 and 2 while the maximum subsidence of 0.427 ft is over Panel 2 (S-46).
2. Only monuments over the Experimental and Waste Panel areas show any appreciable subsidence rate with the higher rates located directly over the Waste Panels. The highest subsidence rates measured for the 2012-2013 surveys correspond to benchmarks located generally over the newer panels (e.g., S-24, S-25, S-29 and S-46) with a maximum rate of  $-5.8 \times 10^{-3}$  m/yr.
3. The effects of subsidence extend away from the repository footprint approximately 1,000 to 1,500 ft (e.g., S-26, see Figures 2.2 and 2.10).

Furthermore, total subsidence and subsidence rates are small, and are approximately at the resolution level of the survey accuracy. The highest subsidence rates are seen above the mined panels and have increased since the mining of Panels 4 through 7. Based on the 2012 survey data, one benchmark exceeded the  $1 \times 10^{-2}$  m/yr TV. This benchmark's rate is  $-1.1 \times 10^{-2}$  m/yr which is slightly greater than the TV. However, the 2013 survey data show the maximum rate for all of the benchmarks are now below the TV. The TV was based on several analyses of potential subsidence over the WIPP site. The largest rate was chosen from these analyses and is

derived from the total subsidence calculated over a 35 year period. The TV assumes that this rate is constant over a 35 year period whereas the monitoring data shows a gradual increase in rate over 20 years. Since only one benchmark has had a subsidence rate approximately at the expected constant rate, the system is responding at a lower rate than the subsidence analysis predicted. As stated earlier, that benchmark is now below the TV subsidence rate. For this reason, no additional analysis is recommended at this time.

## 2.3 Hydrological COMPs

As stated in the previous sections, the CCA lists 10 monitoring parameters that the DOE is required to monitor and assess during the WIPP operational period (DOE 1996). Two of these parameters are considered hydrological in nature and include:

- Changes in Culebra Groundwater Composition
- Changes in Culebra Groundwater Flow

The Scientific Advisor has reviewed the data collected by the MOC during 2011 under the *Strategic Plan for Groundwater Monitoring at the Waste Isolation Pilot Plant (GMP)* (DOE 2003), which comprises two components:

- The Water Quality Sampling Program (WQSP)
- The Water-Level Monitoring Program (WLMP)

WQSP and WLMP data are reported in the Waste Isolation Pilot Plant Annual Site Environmental Report (ASER) for 2013 (DOE 2014e). Additionally, WLMP data are also reported in monthly memoranda from the MOC to the Scientific Advisor.

### 2.3.1 Changes in Culebra Water Composition

#### 2.3.1.1 Water Quality Sampling Program (WQSP)

Table 2.13 summarizes data and TV information relating to the COMP parameter Change in Culebra Water Composition, as well as its implementation in PA.

Under the current WQSP, six wells are sampled by the MOC, all completed to the Culebra Dolomite Member of the Rustler Formation (Figures 2.11 and 2.12). All the WQSP wells are located within the WIPP Land Withdrawal Boundary (LWB). WQSP-1, 2, and 3 are situated hydraulically up-gradient (north) of the WIPP surface facilities and WQSP-4, 5, and 6 are situated down-gradient (south) of the WIPP surface facilities. Previously, the Dewey Lake Formation well WQSP-6A was also sampled, but beginning with sampling round 32, this well is no longer included and is therefore no longer discussed in this section.

The Culebra is modeled for PA because it is the most transmissive, lowest freshwater hydraulic head, saturated water-bearing zone in the WIPP vicinity. Because of this, it is considered the most likely groundwater release pathway for potential future inadvertent human intrusion of the repository. The Culebra is not a source of drinking water for humans and therefore water quality degradation is not of concern. Understanding Culebra water quality is important because it is a key component in understanding the entire flow system.

**Table 2.13 Change in Groundwater Composition - 2014:**

<b>COMP Title:</b>	Groundwater Composition			
<b>COMP Units:</b>	mg/L			
<b>Related Monitoring Data</b>				
Monitoring Program	Monitoring Parameter ID	Characteristics (e.g., number, observation)	Compliance Baseline Value	
Groundwater Monitoring	Composition	Annual chemical analysis	RCRA Background Water Quality Baseline	
<b>COMP Derivation Process – Data acquired in one round (round 35) 2014</b>				
Annually evaluate ASER data and compare to previous years and baseline information				
<b>Related Performance and Compliance Elements</b>				
Element Title	Type & ID	Derivation Procedure	Compliance Baseline	Impact of Change
Groundwater conceptual model, brine chemistry, actinide solubility	Indirect	Conceptual models	Indirect – The average Culebra brine composition is not used.	Provides validation of the various CRA models, potentially significant with respect to flow, transport, and solubility and redox assumptions.
<b>Monitoring Data Trigger Values</b>				
Monitoring Parameter ID	Trigger Value	Basis		
Change in Culebra groundwater composition	Both duplicate analyses for any major ion falling outside the 95% confidence interval (see Table 2.14) for three consecutive sampling periods	The 95% confidence interval for a particular analyte defines the range of concentrations that 19 out of 20 analyses, on average, should fall within. Therefore, TVs should not be set so that a single analysis falling outside the 95% confidence interval is significant. In addition, analysis of solutes in the concentrated brines of the Culebra is not a routine procedure, and occasional analytical errors are to be expected, particularly when a new laboratory is contracted to perform the analyses (SNL 2002).		

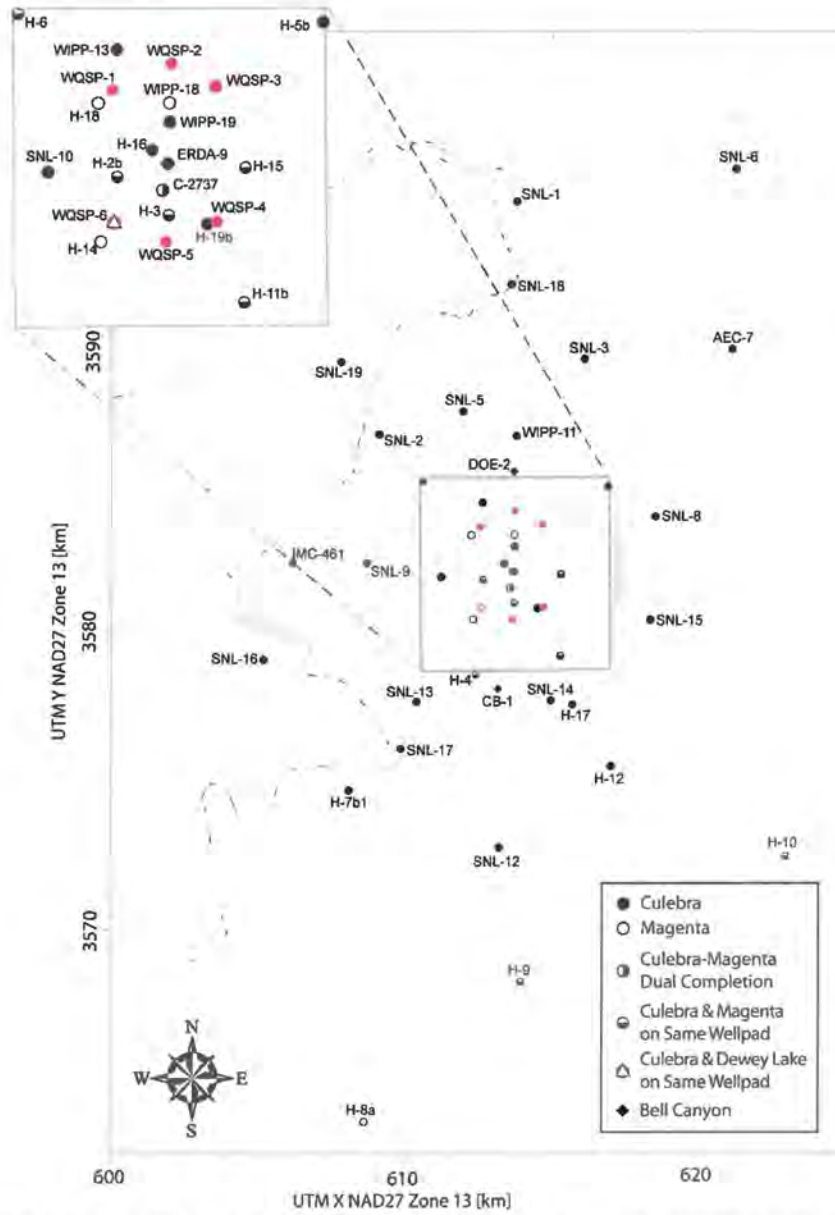


Figure 2.11. Map showing locations of WQSP wells (red) in relation to the WIPP LWB and the rest of the groundwater-monitoring network.

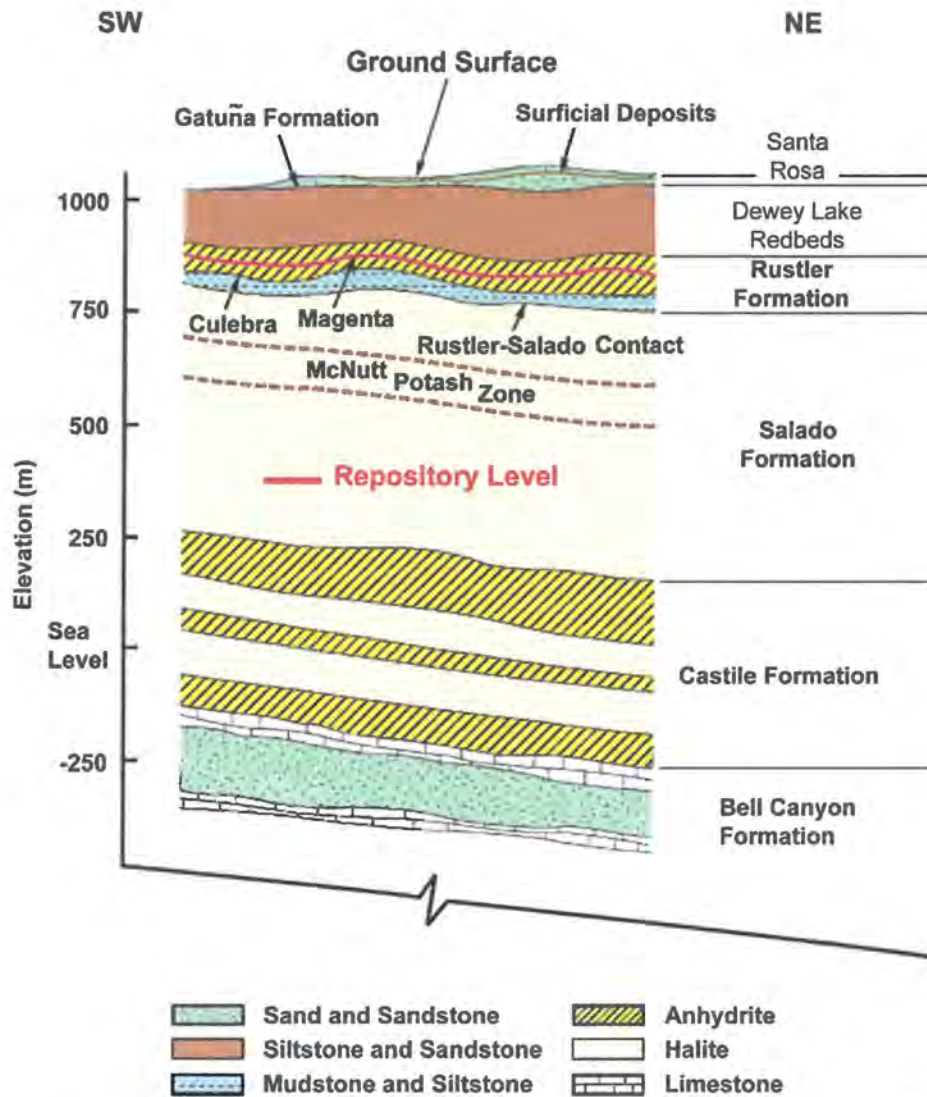


Figure 2.12. Generalized Stratigraphic Cross Section at the WIPP Site.

Solute concentrations in Culebra waters differ widely among wells across the WIPP site, reflecting local equilibrium, diffusion, and, perhaps most importantly, slow regional transport rates. The conceptual model for the Culebra was presented in the CRA-2014 (Appendix TFIELD: DOE 2014b) and is implemented in the PA hydrological models. The conceptual model consists of a confined groundwater flow system with natural-gradient solute travel times across the WIPP site on the order of thousands to tens of thousands of years. In such a system, no changes in water quality at an individual well outside the range of normal analytical uncertainty and noise are expected. If sustained, representative, and statistically significant changes in the concentrations of major ionic species ( $\text{Na}^+$ ,  $\text{Ca}^{2+}$ ,  $\text{Mg}^{2+}$ ,  $\text{K}^+$ ,  $\text{Cl}^-$ ,  $\text{SO}_4^{2-}$ ,  $\text{HCO}_3^-$ ) are observed, this condition could imply that groundwater movement through the Culebra is quicker than what is predicted by the PA models. Stability of major ion concentrations, on the other hand, is consistent with and supports the Scientific Advisor's Culebra transport conceptual model. Thus, this evaluation of the water-quality data focuses on the stability of major ion concentrations.

### **2.3.1.1.1 Water Quality Sampling**

Two water samples (a primary and a duplicate) were collected from each WQSP well currently once per year. Round 35 is the second year of annual sampling; rounds 2–33 (1996–2011) included two sampling rounds per year. Water sampling procedures are outlined in the GMP (DOE 2003) and are summarized here.

Serial and final samples are collected using a submersible pump (each well has its own dedicated pump) set mid-formation. Serial samples are collected at regular intervals during pumping and they are analyzed in a mobile field laboratory to determine when water chemistry has stabilized. Stabilization parameters include temperature, pH, specific gravity and specific conductance. Final samples are collected in the appropriate containers for each particular analysis when water quality parameters have stabilized to within  $\pm 5\%$  of their field parameter averages. Once collected, final samples are placed in coolers and delivered to the analytical laboratory within a day of collection.

### **2.3.1.1.2 Laboratory Analysis**

The MOC collects samples to be analyzed for volatiles, total organic halogens, total organic carbon, semi-volatiles, metals, and general chemistry. For this report, only the results from the metals and general chemistry analyses are discussed, as they provide the necessary information for assessment of the COMP. In the field, the general chemistry samples are not preserved, metals samples are preserved with nitric acid, and neither sample is filtered. In the lab, samples are analyzed using a variety of published, lab-standard methods. Samples are analyzed for major cations (including  $\text{Na}^+$ ,  $\text{Ca}^{2+}$ ,  $\text{Mg}^{2+}$  and  $\text{K}^+$ ), major anions (including  $\text{Cl}^-$ ,  $\text{SO}_4^{2-}$  and  $\text{HCO}_3^-$ ), and other constituents not discussed here.

For sampling rounds 7 through 26, TraceAnalysis, Inc. of Lubbock, TX was responsible for analysis of the water samples submitted by the MOC. In 2008, the analytical contract was awarded to Hall Environmental Analysis Laboratory (HEAL) of Albuquerque, NM, who began analysis with round 27.

### **2.3.1.1.3 Data Analysis**

The results of the WQSP analyses are compared to baseline results in order to determine stability, where concentration of a given ion remains within its baseline-established 95% confidence interval (CI; mean  $\pm$  two standard deviations). Confidence interval calculations assume concentrations follow a normal distribution. The original baseline included the initial five rounds of WQSP well sampling conducted between July 1995 and September 1997 (Crawley and Nagy 1998). The baseline was revised in 2000, expanding from the first five to the first ten rounds of sampling, which were performed between July 1995 and May 2000 (the first receipt of RCRA-regulated waste at WIPP). The baseline data are presented in the *WIPP Resource Conservation and Recovery Act Background Groundwater Quality Baseline Report* (Crawley and Nagy 1998) and in Addendum 1 to that report (IT Corporation 2000). For the purposes of this evaluation, a small number of measurements have been eliminated from the baselines for WQSP-3, 5, and 6. The reasons for eliminating these values are discussed in detail in the COMP's assessment report for data collected in the year 2000 (SNL 2001). The elimination of these values is always conservative; it reduces the "stable" range of concentrations



for the affected parameters. The 95% CIs derived from the baseline data (SNL 2002) are presented in Table 2.14.

Using the baseline analysis described above, a Trigger Value (TV) for Culebra groundwater composition has been defined. The TV occurs when both primary and duplicate analyses for any major ion fall outside the 95% CI for three consecutive sampling periods. Should the TV be reached, the project will first evaluate the sampling and analytical procedures to ensure the adequacy of the sampling. If the change appears to accurately reflect Culebra conditions, the Scientific Advisor will investigate what effects the changes might have on Culebra model conceptualization. The model will be revised to be consistent with the new information if appropriate.

**Table 2.14. Round 35 major ion concentrations and charge-balance errors, with a baseline 95% CI defined for each major ion.**

Well	Round 35	Cl <sup>-</sup> (mg/L)	SO <sub>4</sub> <sup>2-</sup> (mg/L)	HCO <sub>3</sub> <sup>-</sup> (mg/L)	Na <sup>+</sup> (mg/L)	Ca <sup>2+</sup> (mg/L)	Mg <sup>2+</sup> (mg/L)	K <sup>+</sup> (mg/L)	CBE (%)
WQSP-1	primary	36200	4880	50.3	19700	1860	1130	515	-3.1
	duplicate	39500	4910	50.3	19500	1840	1120	534	-7.6
	diff %	-8.7	-0.6	0.0	1.0	1.1	0.9	-3.6	
	CI	31100-39600	4060-5600	45-54	15900-21100	1380-2030	939-1210	322-730	
WQSP-2	primary	<i>36100</i>	5500	47.2	20400	1400	1020	463	-3.7
	duplicate	<i>32100</i>	5190	47.6	20100	1450	1060	485	1.5
	diff %	<i>11.7</i>	5.8	-0.8	1.5	-3.5	-3.8	-4.6	
	CI	31800-39000	4550-6380	43-53	14100-22300	1230-1770	852-1120	318-649	
WQSP-3	primary	127000	<b>8090</b>	32.8	75300	1440	2300	<b>1510</b>	-2.4
	duplicate	129000	<b>9100</b>	32.0	76600	1380	2190	<b>1410</b>	-2.8
	diff %	-1.6	<i>-11.8</i>	2.5	-1.7	4.3	4.9	6.8	
	CI	114000-145000	6420-7870	23-51	62600-82700 <sup>c</sup>	1090-1620	1730-2500	2060-3150 <sup>a</sup>	
WQSP-4	primary	<i>62900</i>	6960	38.7	34700	1630	1150	<b>701</b>	<b>-6.0</b>
	duplicate	<i>56100</i>	7010	39.0	34800	1630	1180	<b>717</b>	-0.5
	diff %	<i>11.4</i>	-0.7	-0.8	-0.3	0.0	-2.6	-2.3	
	CI	53400-63000	5620-7720	31-46	28100-37800	1420-1790	973-1410	832-1550 <sup>b</sup>	
WQSP-5	primary	14300	5000	45.5	9660	1010	424	313	0.5
	duplicate	14700	5030	45.6	9790	1040	427	337	0.1
	diff %	-2.8	-0.6	-0.2	-1.3	-2.9	-0.7	-7.4	
	CI	13400-17600	4060-5940	42-54	7980-10400 <sup>c</sup>	902-1180	389-535	171-523	
WQSP-6	primary	<b>5010</b>	4640	48.2	4260	675	199	147	0.1
	duplicate	<b>4720</b>	4610	47.9	4280	671	201	150	2.2
	diff %	6.0	0.6	0.6	-0.5	0.6	-1.0	-2.0	
	CI	5470-6380 <sup>c</sup>	4240-5120 <sup>c</sup>	41-54	3610-5380 <sup>c</sup>	586-777	189-233 <sup>c</sup>	113-245	

**Bold** denotes analyses returning values outside the 95% CI or a charge-balance error ≥5%

*Italics* denotes sample and duplicate analyses differ by >10%

<sup>a</sup> baseline defined from rounds 8-10

<sup>b</sup> baseline defined from rounds 7-10

<sup>c</sup> baseline definition excludes anomalous values

In addition to the comparison of species concentrations with trigger values, a charge-balance error (CBE) was also calculated for each analysis. The CBE is defined as the difference between the positive and negative charges from the ions in solution divided by the sum of the positive and negative charges. CBE is useful in evaluating analysis reliability because water must be electrically neutral. CBE is rarely zero because of inherent inaccuracy in analytical procedures, but a reliable analysis should not have a CBE exceeding ±5% (Freeze and Cherry 1979). A CBE in excess of ±5% implies either the analysis of one or more ions is inaccurate, or a significant ion has been overlooked. The variation between the results of primary and duplicate sample analysis for each individual ion is also considered. Generally speaking, this variation should be <10%;

large variability can indicate a problem with one or both analyses. Analytical results and CBE for round 35 are presented in Table 2.14.

### **2.3.1.2 Results**

WQSP results for sampling round 35 conducted in 2012 are reported in the 2013 ASER (DOE 2014e, NWP 2014). The reported major ion concentrations are listed in Table 2-14. Sampling round 35 was conducted between March and May; samples were analyzed by HEAL. In the following subsections, anomalous values given in Table 2.14 are identified with either bolded or italicized fonts.

#### **2.3.1.2.1 WQSP-1**

All constituents were within their respective 95% CI, but the CBEs was -7.6% for the duplicate sample. Samples and duplicates were within 10% of each other for all constituents.

#### **2.3.1.2.2 WQSP-2**

All constituents were within their respective 95% CI, and the CBEs were < 5% in absolute value. The chloride primary and duplicate samples differed by 11.7%.

#### **2.3.1.2.3 WQSP-3**

Sulfate ion concentrations measured in both samples were above the 95% CI. Potassium ion concentrations in both samples were below the 95% CI. The CBEs were < 5% in absolute value. Samples and duplicates were within 10% of each other for all constituents.

The sulfate ion concentration in rounds 28 through 35 have been above the 95% CI of 7,870 mg/L, and therefore above the trigger values. Potassium ion concentrations in rounds 27 through 35 have been below the 95% CI of 2,060 mg/L, and therefore below the trigger values. Sulfate and potassium are minor ions (chloride concentrations in WQSP-3 are almost 18 times higher than sulfate concentrations and almost 88 times higher than potassium concentrations). This deviation is not a significant event warranting further investigation at this time. These trigger values will likely be revised, based upon analysis of all sampling rounds to date.

#### **2.3.1.2.4 WQSP-4**

Both samples' potassium ion concentrations were below the 95% CI. The CBE in for the primary sample was -6.0%. The chloride primary and duplicate samples differed by 11.4%.

The potassium ion concentration in rounds 27 through 35 were all below the lower 95% CI of 832 mg/L, and therefore are below the trigger value. Potassium is one of the minor cations (sodium concentrations in WQSP-4 are over 40 times higher), and this deviation is not a significant event warranting further investigation at this time. This trigger value will likely be revised, based upon analysis of all sampling rounds to date.

#### **2.3.1.2.5 WQSP-5**

All constituents were within their respective 95% CI, and the CBEs were < 5% in absolute value. Samples and duplicates were within 10% of each other for all constituents.

### 2.3.1.2.6 WQSP-6

Chloride ion concentrations measured in both samples were above the 95% CI. Samples and duplicates were within 10% of each other for all constituents, and the CBEs were < 5% in absolute value.

### 2.3.1.3 Assessment of Culebra Water Quality Data

Two of the 12 calculated CBEs had absolute values >5%. The analyses with larger CBEs are negative (more anions than cations). A common method of assessing water-quality stability is through the use of Piper diagrams, which illustrate relative proportions of three cation and three anion concentrations (four cations are treated by lumping sodium and potassium together). By plotting the ion ratios for every round (including outliers thrown out in the calculation of confidence intervals presented in Table 2.14), we can visually assess water quality trends. Piper diagrams of Culebra water chemistry (Figure 2.13) over the course of the WQSP (now 15+ years) show that the groundwater is quite stable, with results for each well continually plotting within relatively small envelopes.

The Piper diagrams illustrate that WQSP-3 and WQSP-4 do not show significant deviation, even though the potassium ion concentrations have been below the lower 95% C.I. for multiple sampling rounds. This is due to the small contribution that the potassium ion has to overall water chemistry.

An assessment of the Culebra water-chemistry results show that the water chemistry in the WQSP wells is stable and that these wells only have two minor analytes ( $\text{SO}_4^{2-}$  and  $\text{K}^+$ ) that are in violation of a TV. Based on review of CBEs calculated for each WQSP well sampled, the analytical results appear to be generally reliable, although CBE are consistently negative. Any variability observed in the data suggesting instability can be attributable to analytical problems. As mentioned in a previous COMPs report (SNL 2012), it is believed that the majority of analytical problems can be linked to the high salinity (i.e., TDS) observed in Culebra brines. The sensitive analytical equipment used in environmental labs requires that samples be diluted up to 10,000 times in order for samples to be run without harming the machine. Dilution of the samples introduces both human and analytical error, which can cause results to be less precise, especially for constituents that make up a small portion of the overall charge balance.

The trigger values used here were computed based only on samples collected before WIPP opened (2000 and before). Sandia will revise trigger values in light of the 68 samples collected from each well since 1995 (rounds 1-34). The analysis will adjust the confidence intervals (used as TVs) to more accurately represent the variability and stability of the Culebra groundwater, based upon more than 15 years of sample data. The results of the analysis and the justifications for any changes to the TVs will be discussed in a revision to the Trigger Derivation Report (Wagner and Kuhlman 2010) and in next-year's COMPs report.

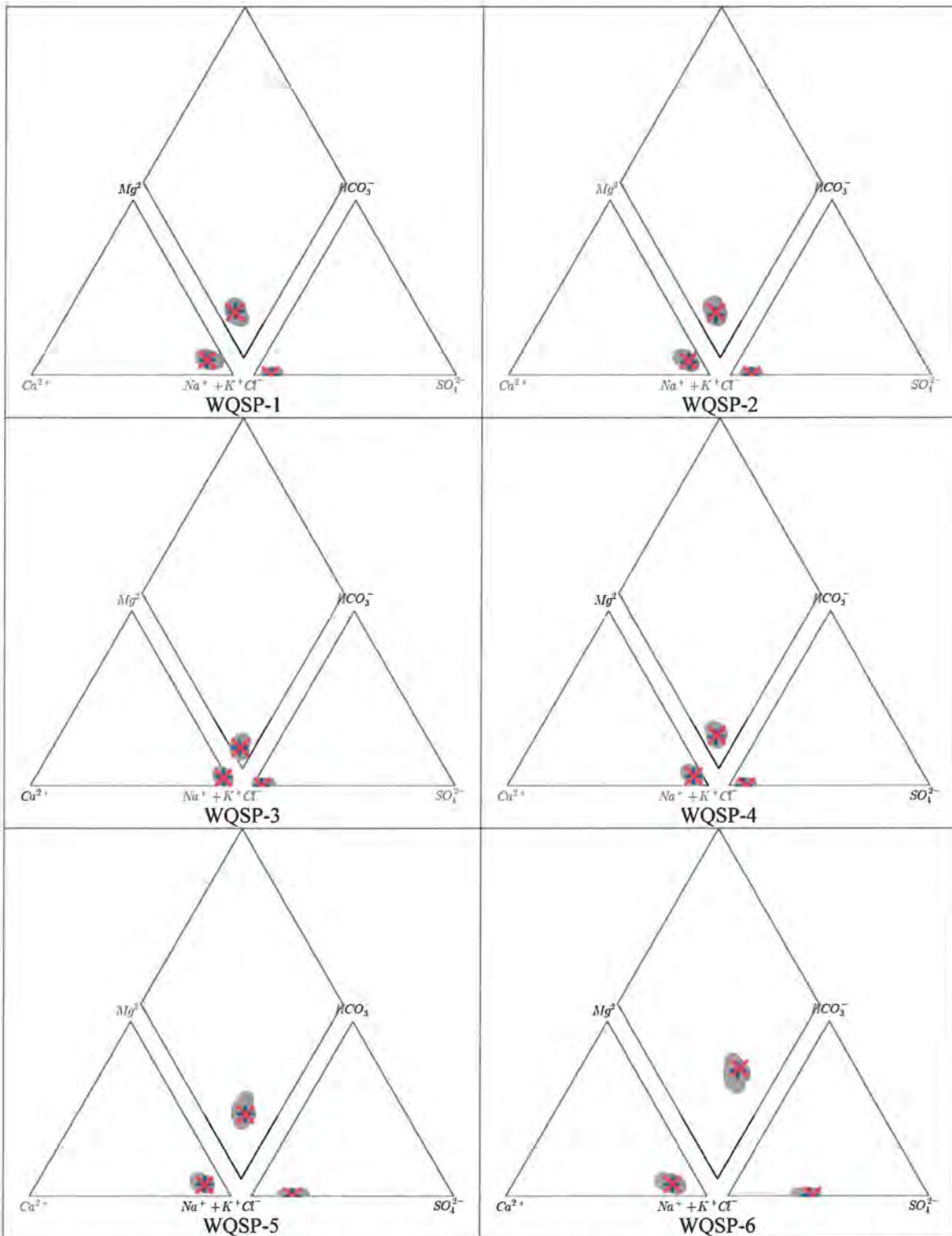


Figure 2.13. Piper diagrams of all data collected from WQSP-1 through WQSP-6. The plots show both historical data (gray areas) and results from rounds 35 (primary = blue "+" and duplicate = red "x").

### **2.3.2 Changes in Groundwater Flow (Water Level)**

Table 2.15 summarizes data and TV information relating to the COMP parameter Change in Groundwater Flow, as well as its implementation in PA. Assessment of the COMP for the Culebra involves comparisons of two sets of modeling results. The baseline model results are derived from the ensemble of models used in PA for CRA-2009 PABC (e.g., Hart et al., 2009; Kuhlman, 2010a), while annual model results are adjusted to best fit freshwater heads observed in 2013 (DOE 2014e).

The Dewey Lake, Magenta, and Bell Canyon are not currently monitored as COMPs, do not have PA flow models, and therefore do not have TVs. The water-level measurements in these units do, however, provide information used in the development of the conceptual model of overall site hydrology.

#### **2.3.2.1 Water Level Monitoring Program (WLMP)**

In 2013, the MOC made monthly water-level measurements in all of the WIPP non-shallow subsurface water (SSW) monitoring network wells (see Figure 2-14 and Table 2.16), or quarterly in any redundant wells (i.e., six of the seven H-19b wells). As of February 2013, the WIPP monitoring network consisted of 65 wells (including one dual-completion Magenta-Culebra well), see Table 2-16. There were 49 wells with completions to the Culebra Member of the Rustler Formation, 13 to the Magenta Member of the Rustler Formation, two to the Bell Canyon Formation, and one to the Dewey Lake Formation. Since the last COMPs report, the replacement well AEC-7R was drilled to the Culebra on the same well pad as AEC-7, which was plugged and abandoned. AEC-7 was plugged and abandoned after the February 2013 water level was collected.

**Table 2.15 Changes in Groundwater Flow - 2014:**

<b>COMP Title:</b>		Changes in Culebra Groundwater Flow		
<b>COMP Units:</b>		Inferred from water-level data		
<b>Related Monitoring Data</b>				
Monitoring Program	Monitoring Parameter ID	Characteristics (e.g., number, observation)	Compliance Baseline Value	
Groundwater Monitoring	Head and Topography	Monthly water-level measurements, annual pressure-density surveys.	Indirect	
<b>COMP Derivation Procedure - Data acquired between December 2012 and December of 2013</b>				
Annual assessment from ASER data.				
<b>Related PA Elements</b>				
Element Title	Type & ID	Derivation Procedure	Compliance Baseline	Impact of Change
Groundwater conceptual model, Transmissivity fields	T-Fields	Computer codes are used along with groundwater data to generate transmissivity fields for the Culebra on a regional scale. A summary of the conceptualization, implementation and calibration of the Culebra T-fields is given in Kuhlman (2010b).	Attachment T-FIELDS to Appendix PA.	Validates assumptions used in T-Field modeling and the groundwater basin model.
<b>Monitoring Data Trigger Values</b>				
Monitoring Parameter ID	Trigger Value	Basis		
Change in Culebra Groundwater Flow	PA Compliance Baseline	Model-predicted travel time in the Culebra is compared to the distribution found in PA, for an ensemble-average model with best-fit boundary conditions to the current year's observed freshwater heads. The travel time from the center of the WIPP panels to the WIPP LWB must fall within the distribution found using 100 model runs used in the baseline PA.		

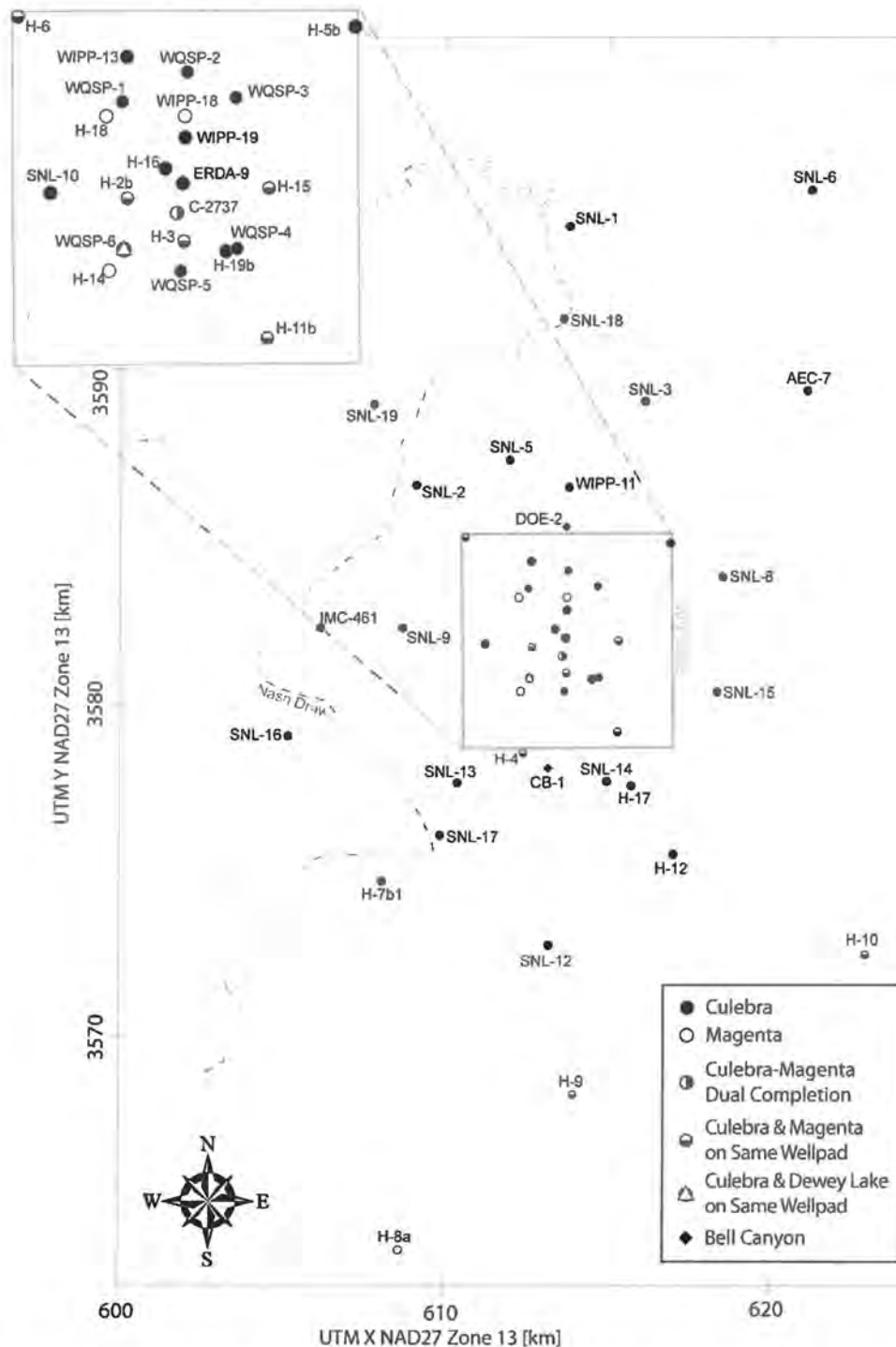


Figure 2.14. Map of the WIPP area showing well pad locations discussed in this section (See Table 2.16 for listing of wells at each well pad).

**Table 2.16 February 2013 Non-SSW<sup>1</sup> WIPP Groundwater Monitoring Network**

Well	Pad <sup>2</sup>	Completion <sup>3</sup>
AEC-7R	AEC-7R	CUL
C-2737	C-2737	CUL/MAG DUAL
CB-1	CB-1	BC
DOE-2	DOE-2	BC
ERDA-9	ERDA-9	CUL
H-2b1	H-2b	MAG
H-2b2		CUL
H-3b1	H-3	MAG
H-3b2		CUL
H-4b	H-4	CUL
H-4c		MAG
H-5b	H-5b	CUL
H-6bR	H-6	CUL
H-6c		MAG
H-7b1	H-7b1	CUL
H-8a	H-8a	MAG
H-9bR	H-9	CUL
H-9c		MAG
H-10a	H-10	MAG
H-10c		CUL
H-11b2	H-11b	MAG
H-11b4		CUL
H-12	H-12	CUL
H-14	H-14	MAG
H-15R	H-15	CUL
H-15		MAG
H-16	H-16	CUL
H-17	H-17	CUL
H-18	H-18	MAG

Well	Pad <sup>2</sup>	Completion <sup>3</sup>
H-19b0	H-19b	CUL
H-19b2		CUL REDUN
H-19b3		CUL REDUN
H-19b4		CUL REDUN
H-19b5		CUL REDUN
H-19b6		CUL REDUN
H-19b7		CUL REDUN
IMC-461	IMC-461	CUL
SNL-1	SNL-1	CUL
SNL-2	SNL-2	CUL
SNL-3	SNL-3	CUL
SNL-5	SNL-5	CUL
SNL-6	SNL-6	CUL
SNL-8	SNL-8	CUL
SNL-9	SNL-9	CUL
SNL-10	SNL-10	CUL
SNL-12	SNL-12	CUL
SNL-13	SNL-13	CUL
SNL-14	SNL-14	CUL
SNL-15	SNL-15	CUL
SNL-16	SNL-16	CUL
SNL-17	SNL-17	CUL
SNL-18	SNL-18	CUL
SNL-19	SNL-19	CUL
WIPP-11	WIPP-11	CUL
WIPP-13	WIPP-13	CUL
WIPP-18	WIPP-18	MAG
WIPP-19	WIPP-19	CUL
WQSP-1	WQSP-1	CUL
WQSP-2	WQSP-2	CUL
WQSP-3	WQSP-3	CUL
WQSP-4	WQSP-4	CUL
WQSP-5	WQSP-5	CUL
WQSP-6	WQSP-6	CUL
WQSP-6a		DL

<sup>1</sup> SSW wells and piezometers monitor the Santa Rosa / Dewey Lake Formation contact at the WIPP facilities

<sup>2</sup> Pad names used in Figure 2.14

<sup>3</sup> Well completions codes are as follows:

- CUL: Culebra Member of the Rustler Formation
- MAG: Magenta Member of the Rustler Formation
- BC: Bell Canyon Formation
- DL: Dewey Lake Formation
- DUAL: dual-completion well
- REDUN: redundant well (quarterly water levels)



### 2.3.2.2 Culebra Groundwater Flow Results and Assessment

Assessment of Culebra data involves the interpretation of freshwater head data in the context of the hydrogeologic knowledge about the WIPP area. If heads change significantly in wells, this may be due to an underlying change in flow Culebra flow patterns. At the request of the New Mexico Environment Department (NMED), the Scientific Advisor uses the ensemble-average of the 100 calibrated Culebra groundwater flow model runs developed for PA to create the baseline transmissivity (T) field. This ensemble-average T field is used to produce the freshwater head potentiometric surface map each year for the ASER. Each year the boundary conditions of the ensemble-averaged model are adjusted to best fit the observed freshwater head values from that year. The ensemble-averaged T field and the adjusted boundary conditions are used as inputs to the MODFLOW model (Harbaugh et al. 2000) that computes the heads which are then contoured and presented in the ASER.

The Culebra PA model is a single-layer groundwater flow model that incorporates information about aquifer parameters (e.g., T, storativity, and anisotropy) and is based upon a peer-reviewed conceptual model of Culebra geology (Section 8.2 of EPA 2010b). The model is calibrated to both steady-state freshwater head and transient pumping test drawdown data. The contour map shown in Figure 2.17 shows the area immediately around the WIPP land withdrawal boundary, and indicates that flow is generally from north to south, which is consistent with previous results, and that the gradient is steepest across the area including the WIPP surface facilities, caused by a region of low Culebra T.

The contour map is created according to SNL specific procedure SP 9-9, and the results of following the procedure along with detailed narrative descriptions are given in the analysis report *Analysis Report for Preparation of 2013 Culebra Potentiometric Surface Contour Map* (Kuhlman 2014). This material is summarized in the 2013 ASER, Section 6.2.5 (DOE 2014e).

### 2.3.2.3 Culebra Freshwater-Head Results and Assessment

Table 2.17 shows the February 2013 freshwater heads reported in the 2013 ASER and used in the development of the Culebra contour map given in the 2013 ASER (DOE 2014e). The particle shown as a blue arrow in Figure 2.16 begins where the Culebra intersects the WIPP waste-handling shaft and continues to the WIPP LWB, as required by NMED. The travel time for this particle in the boundary-calibrated ensemble-average flow field (5,964 years) is compared to the distribution of 100 travel times computed for the CRA-2009 PABC. The fastest travel time from the ensemble of 100 fields is less than 3,000 years (see red dots in Figure 2.15), therefore the ensemble-average travel time falls inside the predicted CRA-2009 PABC range. The particles illustrated in Figures 2.16 and 2.17 are released from the point in the Culebra corresponding to the center of the WIPP waste panels underground (the same location as well C-2737).

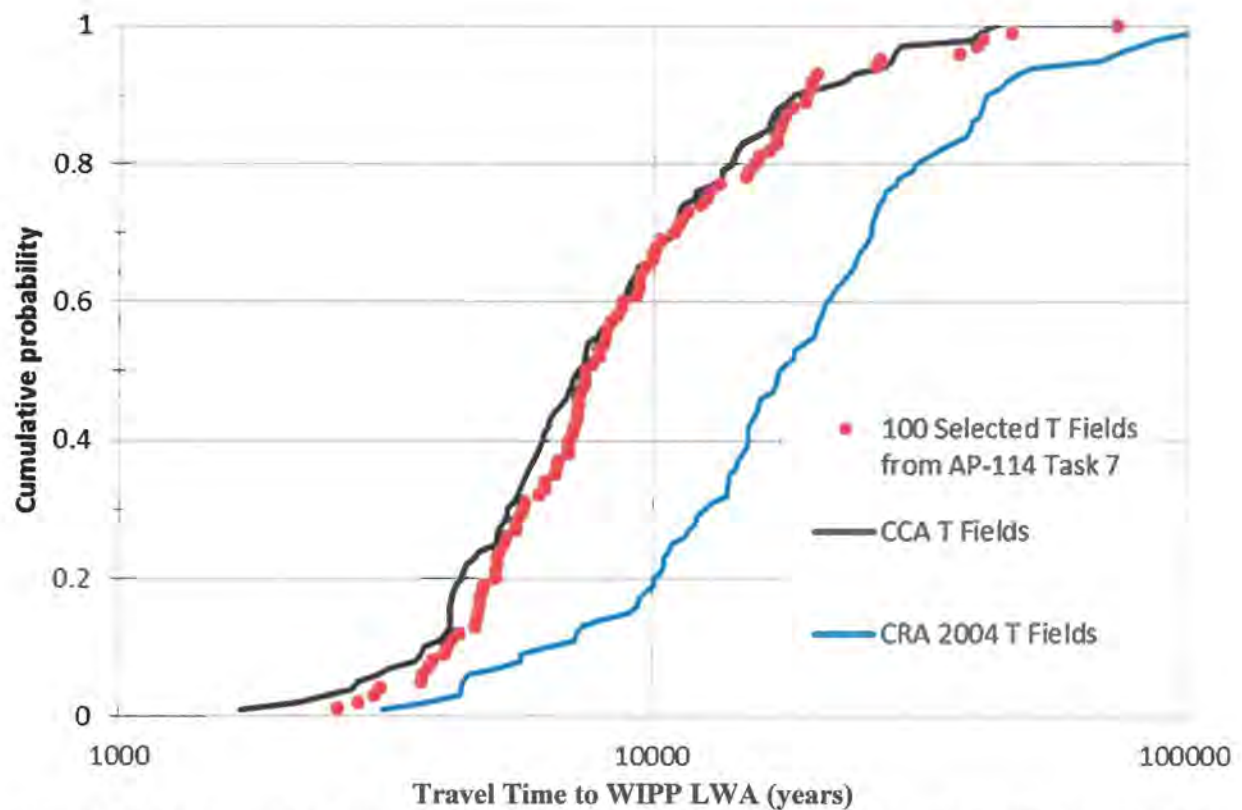


Figure 2.15. Distribution of Particle Travel Times from C-2737 (Center of Waste Panels) to WIPP LWB for CCA (black line), CRA-2004 (blue line), and CRA-2009 PABC (red dots). Figure from Hart et al. (2009).

In Universal Transverse Mercator, North American Datum of 1927 (UTM NAD27) Zone 13 coordinates (meters), the waste-handling shaft is located at the (X, Y) location (613579, 3582079), while the center of the waste panels is (613597, 3581401). The distance between these two points is 678 meters, mostly in the north-south direction; the difference can be seen by comparing the location of the tail of the blue arrow and the location of C-2737 in Figure 2.17. The particle trace in the ensemble-average flow field has a length of 4,092 meters.

The ensemble average transmissivity (T) field used to compute the contour map for the ASER is by construction much smoother than any of the 100 stochastically generated fields it is averaged from. This smoothness of the input T field results in a smoother and relatively faster particle trace; compare the particle traces in Figure 2.17 (smoothed average field) and Figure 2.18 (original T fields from PA).

### Freshwater Heads WIPP Area 2013

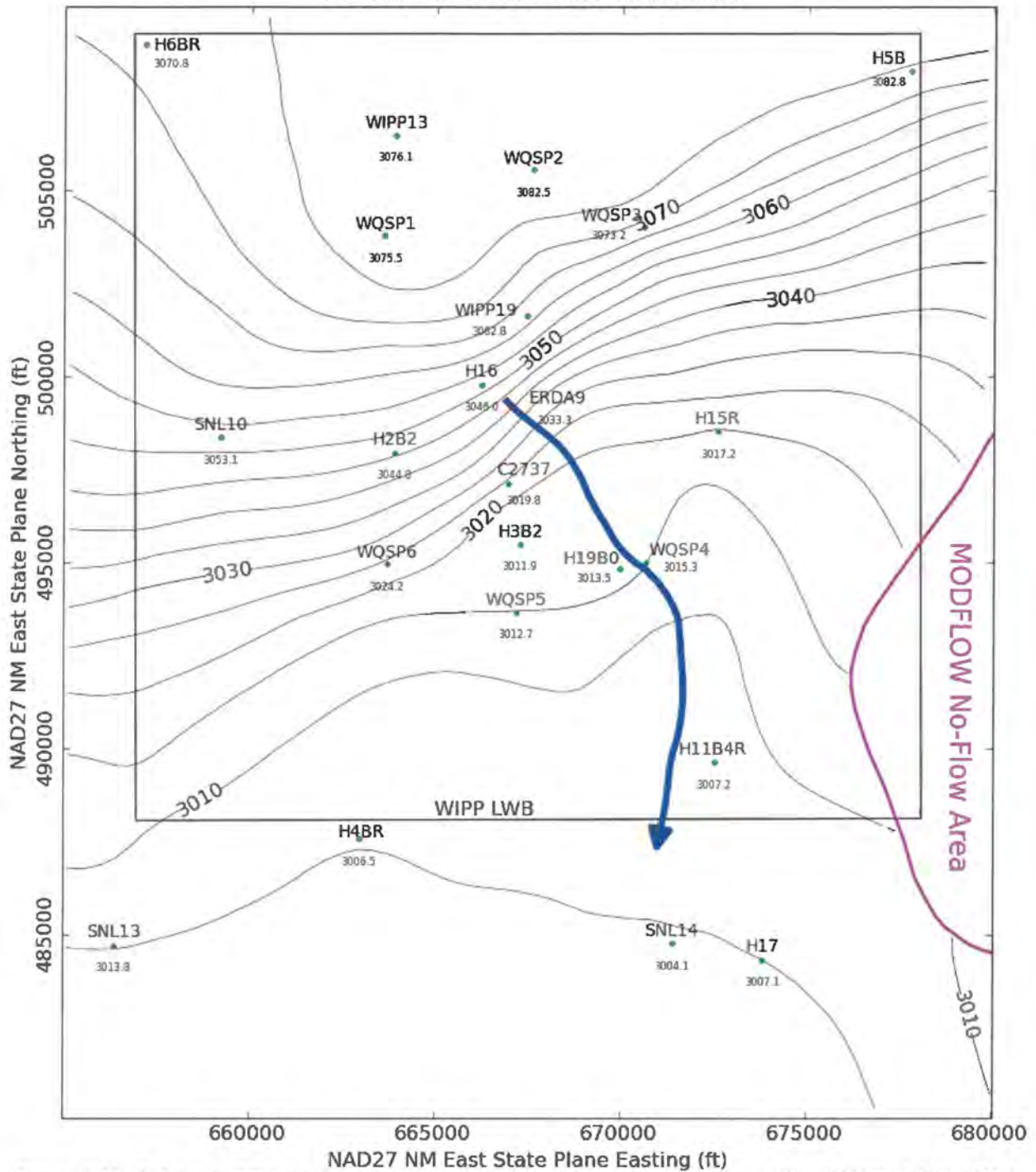


Figure 2.16. February 2013 modeled Culebra potentiometric surface of the immediate WIPP vicinity (DOE 2014e) generated using ensemble average distributed aquifer parameters from the SNL Culebra flow model used in performance assessment baseline calculation for CRA-2009; see Kuhlman (2010b).

**Table 2.17. Summary of February 2013 Culebra freshwater heads.**

Culebra Well <sup>2</sup>	Measurement Date	Adjusted Freshwater Head [m AMSL]	Specific Gravity
AEC-7	02/08/13	933.39	1.067
C-2737 (PIP <sup>1</sup> )	02/12/13	920.45	1.023
ERDA-9	02/12/13	924.54	1.073
H-02b2	02/12/13	928.05	1.012
H-03b2	02/12/13	918.01	1.036
H-04bR	02/11/13	916.38	1.017
H-05b	02/08/13	939.65	1.095
H-06bR	02/11/13	935.97	1.038
H-07b1	02/07/13	913.92	1.007
H-09bR	01/07/13	912.99	1.000
H-10c	02/08/13	923.85	1.094
H-11b4R	02/11/13	916.61	1.076
H-12	02/11/13	918.46	1.113
H-15R	02/12/13	919.66	1.118
H-16	02/12/13	928.42	1.037
H-17	02/08/13	916.55	1.133
H-19b0	02/11/13	918.51	1.066
IMC-461	02/07/13	926.97	1.000
SNL-01	02/07/13	938.85	1.029
SNL-02	02/07/13	936.48	1.009
SNL-03	02/07/13	938.42	1.028
SNL-05	02/07/13	936.57	1.009
SNL-08	02/08/13	981.74	1.243
SNL-09	02/08/13	930.75	1.094
SNL-10	02/07/13	930.82	1.018
SNL-12	02/07/13	930.60	1.009
SNL-13	02/08/13	915.02	1.006
SNL-14	02/07/13	918.62	1.018
SNL-16	02/08/13	915.65	1.046
SNL-17	02/08/13	920.47	1.229
SNL-18	02/07/13	917.54	1.009
SNL-19	02/08/13	916.02	1.005
WIPP-11	02/07/13	936.49	1.005
WIPP-13	02/07/13	936.49	1.007
WIPP-19	02/12/13	939.03	1.038
WQSP-1	02/12/13	937.58	1.041
WQSP-2	02/12/13	933.54	1.052
WQSP-3	02/12/13	937.42	1.051
WQSP-4	02/12/13	939.53	1.048
WQSP-5	02/12/13	936.70	1.147
WQSP-6	02/11/13	919.07	1.077

<sup>1</sup> PIP (production injection packer) indicates water levels measured in C-2737, a dual-completed well.

<sup>2</sup> SNL-06, and SNL-15 are not listed here because they are currently not representative of undisturbed conditions in the Culebra; water levels in these well are predicted to continue to rise for the foreseeable future.

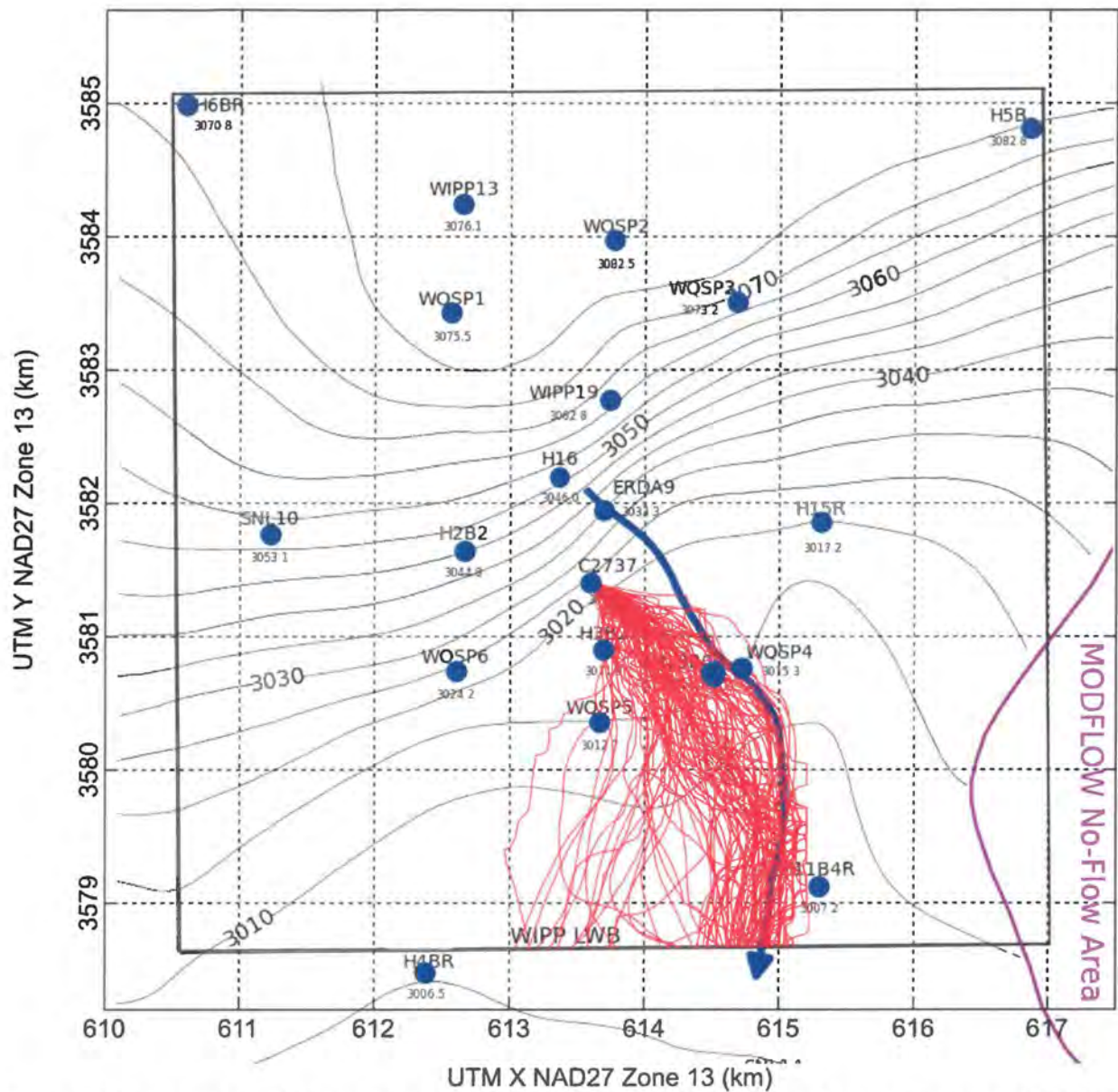


Figure 2.17. Distribution of 100 particle traces (red lines) from C-2737 (center of waste panels) to WIPP LWB (heavy black line) for CRA-2009 PABC. Figure is combination of contours and blue contour from 2013 ASER (DOE 2014e, and individual realization particle traces from CRA-2009 PABC (Kuhlman, 2010a). Culebra monitoring wells are indicated with blue circles.

#### **2.3.2.4 Interpretation/Summary of the 2013 Culebra Data**

As mentioned previously, change in Culebra groundwater flow would be manifested as a change in gradient and/or flow velocity, which would be observed through changes in freshwater head measured in observation wells. In general, the freshwater potentiometric gradient of the Culebra is and has been from north to south and flow velocities are low across the WIPP modeling domain (Hart et al., 2009). The basis of the assessment of the groundwater flow COMP is the computed travel time and potentiometric surface map of the Culebra (Figure 6.11; DOE 2014e). The map was generated using the Culebra flow model developed by the Scientific Advisor for performance baseline calculations associated with CRA-2009 PABC and Culebra heads from February 2013.

The ensemble-model predicted travel time for a particle currently falls within the range modeled for PA, although it is near the faster end of the distribution because of the smoothness of the averaged field, compared to the stochastically generated individual fields used in PA. The travel time indicates that the current observed freshwater heads are consistent with the model used in PA, and therefore they do not violate the TV defined last year.

#### **2.3.2.5 Results and Assessment of Data from Other Units**

Assessment of water-level changes from other hydrologic units present in the WIPP vicinity (Table 2.18) is important for confirming the conceptual model of overall site hydrology. Water-level measurements for the Magenta Member of the Rustler Formation provide information about confinement of and connectivity to the underlying Culebra Member.

For consistency with the time period chosen for reporting Culebra water levels, February 2013 was chosen as the time period for reporting water level data from other (non-Culebra) units. Water-level changes in the Magenta ranged from -0.3 to 1.9 m, with three wells experiencing water-level changes  $\geq 0.61$  m (2.0 ft). Aside from recovery due to pumping and sampling activities conducted by the Scientific Advisor, water levels in wells are largely stable. The water levels in H-02b1 is 1.9 meters higher than 2012 because the wells are recovering from water quality testing activities in early 2011. The water level in H-15 rose 0.8 m since August 2011; it has been rising steadily since the well was re-completed in 2008.

The water level was stable in WQSP-6A. This well is completed to the middle of the Dewey Lake Formation (Table 2.18). Water levels in DOE-2 are stable, while water levels in CB-1 have continued to slowly rise (1.6 m since February 2012). This rise has continued since 2008 swabbing activities cleaned out foreign water and subsequently changed wellbore water densities significantly (Table 2.18).

**Table 2.18. Summary of 2009-2013 water-level changes in units other than the Culebra.**

Well	Dec 2009 Water Level Elevation	Dec 2010 Water Level Elevation	Aug 2011 Water Level Elevation	Feb 2012 Water Level Elevation	Feb 2013 Water Level Elevation	2013-2012 Water Level Change
	(m AMSL)	(m AMSL)	(m AMSL)	(m AMSL)	(m AMSL)	(m)
<i>Magenta Wells</i>						
C-2737	958.06	958.22	958.35	958.42	958.54	0.1
H-02b1	958.24	958.25 <sup>a</sup>	949.69	955.81	<b>957.71</b>	<b>1.9</b>
H-03b1	958.59	958.95	959.12	959.15	959.33	0.2
H-04c	959.49	959.49 <sup>a</sup>	958.78	959.02	959.38	0.4
H-06c	935.8	935.91	936.07	936.09	936.33	0.2
H-08a	922.78	920.37	922.1	922.53	923.29	0.8
H-09c	956.68	956.7 <sup>b</sup>	956.39	956.14	955.84	-0.3
H-10a	981.95	948.6	948.71	948.73	948.73	0.0
H-11b2	956.69	956.78	956.97	956.94	957.14	0.2
H-14	946.77	955.05	956.16	956.57	956.98	0.4
H-15	954.55	955.78	958	958.63	<b>959.42</b>	<b>0.8</b>
H-18	960.08	961.03	961.46	961.68	962.13	0.5
WIPP-18	960.1	959.89	960.11	960.2	960.45	0.3
<i>Dewey Lake Well</i>						
WQSP-6A	974.44	974.36	974.29	974.24	974.24	0.0
<i>Bell Canyon Wells</i>						
CB-1	917.35	918.41	919.39	920.11	<b>921.68</b>	<b>1.6</b>
DOE-2	934.71	934.73	934.88	934.9	935.04	0.1

<sup>a</sup> March 2010 water level; no December water level due to Scientific Advisor sampling activities

<sup>b</sup> September 2010 water level; no December water level due to drilling and plugging activities

**Bold** = absolute changes in water level  $\geq 0.61$  m (2.0 ft)

## 2.4 Waste Activity

Table 2.19 summarizes data and TV information relating to the COMP parameter Waste Activity, and its implementation in PA. The reporting period for the waste activity COMP started at first waste receipt and ended on June 30, 2014. A comparison of the tracked actinides and the total repository inventory used in the PABC-2009 is detailed in Table 2.20. No other activity-related assessment has been made at this time.

There are no TVs for CH activity, only RH. The TV for RH is the regulatory limit of 5.1 million Curies. The total curies of RH waste for the period ending June 30, 2014 is  $2.41 \times 10^4$  Curies, well below the TV. There are no recognized reportable issues associated with this COMP. No

changes to the monitoring program are recommended at this time. A detailed waste inventory assessment has been provided in the CRA-2009 (DOE 2009).



**Table 2.19 Waste Activity - 2014:**

<b>COMP Title:</b>		<b>Waste Activity</b>		
<b>COMP Units:</b>		Curies		
<b>Related Monitoring Data</b>				
<b>Monitoring Program</b>	<b>Monitoring Parameter ID</b>	<b>Characteristics (e.g., number, observation)</b>	<b>Compliance Baseline Value</b>	
Waste Data System (WDS; formerly the WWIS), BIR	Radionuclide activity per container and volume	Curies , volume	TRU Waste Inventory for the 2009 Compliance Recertification Application Performance Assessment Baseline Calculation (Crawford et al. 2008)	
<b>COMP Assessment Process - Reporting Period July 1, 2013 to June 30, 2014</b>				
Total curie content of emplaced CH-TRU and RH-TRU waste. <i>[Total radionuclide inventories reported by the WDS]</i>				
<b>Year 2014 COMP Assessment Value</b>				
A comparison of emplaced and PA waste parameters is found in Table 2.20.				
<b>Element Title</b>	<b>Type and ID</b>	<b>Derivation Procedure</b>	<b>Compliance Baseline</b>	<b>Impact of Change</b>
Radionuclide inventories	Parameter	Product of waste stream content and volume scaled up to the Land Withdrawal Act limits. (U.S. Congress 1992)	Table 5-6 of Crawford et al. 2008	May affect direct brine releases for those radionuclides that become inventory-limited during a PA simulation.
Activity of waste intersected for cuttings and cavings releases.	Parameter	Function of waste stream volumes and activities	Crawford et al. 2008	Cuttings are a significant contributor to releases. An increase in activity of intersected waste is potentially significant.
WIPP-scale average activity for spallings releases	Parameter	Average of all CH-TRU waste only.	Crawford et al. 2008	Spallings are a significant contributor to releases. An increase in average activity of intersected waste is potentially significant.
<b>Monitoring Data Trigger Values</b>				
<b>Monitoring Parameter ID</b>	<b>Trigger Value</b>	<b>Basis</b>		
Waste emplacement records	None	Administrative controls address waste limits. TV Derivation Report, Revision 2 (Wagner and Kuhlman 2010)		
Total emplaced RH-TRU waste activity	5.1 million curies	LWA emplacement limit reached. Administrative controls address these limits.		

**Table 2.20 Comparison of tracked radionuclide inventory to the PABC09 Inventory (from WRES 2014 and Crawford et al. 2008).**

Radionuclide (CCA Table 4-10)	Non-Decayed Total Activity as of June 30, 2013	Non-Decayed CH Inventory as of June 30, 2014	Non-Decayed RH Inventory as of June 30, 2014	Non-Decayed Total Activity as of June 30, 2014	PABC09 Total Inventory at Closure (2033)
<sup>241</sup> Am	2.491E+05	2.579E+05	6.208E+02	2.585E+05	4.72E+05
<sup>137</sup> Cs	1.255E+04	1.421E+01	1.444E+04	1.445E+04	8.95E+04
<sup>238</sup> Pu	4.764E+05	4.828E+05	7.289E+02	4.835E+05	1.47E+06
<sup>239</sup> Pu	3.263E+05	3.333E+05	3.839E+02	3.337E+05	5.13E+05
<sup>240</sup> Pu	8.068E+04	8.239E+04	2.802E+02	8.267E+04	1.45E+05
<sup>242</sup> Pu	2.658E+01	2.720E+01	3.821E-01	2.759E+01	7.59E+01
<sup>90</sup> Sr	6.784E+03	1.595E+01	7.599E+03	7.615E+03	8.04E+04
<sup>233</sup> U	6.740E+00	6.536E+00	3.848E-01	6.921E+00	2.07E+02
<sup>234</sup> U	8.665E+01	8.669E+01	1.140E-01	8.783E+01	3.09E+02
<sup>238</sup> U	1.737E+01	1.758E+01	3.915E-02	1.762E+01	2.73E+01
<b>Total</b>	<b>1.152E+06</b>	<b>1.157E+06</b>	<b>2.405E+04</b>	<b>1.181E+06</b>	<b>2.77E+06</b>

### 3 COMPs Assessment Conclusion

The operational period monitoring program designed to meet the Assurance Requirements of 40 CFR § 191.14 and the terms of WIPP certification was initiated in 1999. This monitoring program is useful to further validate the assumptions and conceptual models that are used to predict WIPP performance and identify conditions that could potentially cause radioactive release above the limits established in 40 CFR § 191.13. Since releases above these limits cannot occur during the operational period of WIPP, the monitoring program looks at other potential performance indicators of the disposal system and compares these data to PA performance expectations. Specifically, 10 monitoring parameters are assessed and compared to PA expectations and assumptions. The CRA-2014 (DOE 2014b) contains the results of the most recent PA submitted to the EPA for compliance purposes. The PABC-2009 was used in EPA's 2010 certification decision and became the new compliance baseline PA (EPA 2010a). The CRA-2014 is under review by EPA and will become the latest baseline when recertification occurs. The results of this year's COMP assessment conclude that there are no COMPs data or results that indicate a reportable event or condition adverse to predicted performance.

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